

**PLEASE REFER TO FILE  
FOLDER NO. 9**

**FOR THE  
CONTINUATION OF THIS  
WATER RIGHT FILE**

**No. 63-32089**

IDAHO DEPARTMENT OF WATER RESOURCES  
WELL DRILLER'S REPORT

Office Use Only		
Well ID No.		
Inspected by		
Twp	Rge	Sec
1/4	1/4	1/4
Lat:	Long:	

1. WELL TAG NO. D 0041980  
DRILLING PERMIT NO. 890994-835987  
Water Right or Injection Well No. 63-32089-63-32090

## 2. OWNER:

Name CITY OF EAGLE  
Address P.O. BOX 1520  
City EAGLE State ID Zip 83616

3. LOCATION OF WELL by legal description: LEGACY WELL #1  
You must provide address or Lot, Blk, Sub, or Directions to well.

Twp. 4 North ☒ or South ☐  
Rgs. 1 East ☐ or West ☒  
Sec. 11 1/4 SE 1/4 NW 1/4  
Gov't Lot \_\_\_\_\_ County ADAMS

Lat: \_\_\_\_\_ Long: \_\_\_\_\_  
Address of Well Site QUARTER CIRCLE IS RANCH, WEST  
OF EAGLE ROAD City EAGLE

L1 \_\_\_\_\_ Blk. \_\_\_\_\_ Sub. Name \_\_\_\_\_

## 4. USE:

☐ Domestic ☐ Municipal ☐ Monitor ☐ Irrigation  
☐ Thermal ☐ Injection ☒ Other TEST

## 5. TYPE OF WORK check all that apply (Replacement etc.)

☒ New Well ☐ Modify ☐ Abandonment ☐ Other \_\_\_\_\_

## 6. DRILL METHOD:

☐ Air Rotary ☐ Cable ☐ Mud Rotary ☒ Other REVERSE

## 7. SEALING PROCEDURES

Seal Material	From	To	Weight / Volume	Seal Placement Method
CEMENT GROUT	0	272	24 gal	PUMPED BOTTOM TO TOP
BENTONITE	360	415	12,000	DRY POUR

Was drive shoe used? ☐ Y ☒ N Shoe Depth(s): \_\_\_\_\_  
Was drive shoe seal tested? ☐ Y ☒ N How? \_\_\_\_\_

8. CASING/LINER: 16" X 12" REDUCER @ 180' TO 181'

Diameter	From	To	Gauge	Material	Casing	Liner	Welded	Threaded
16	0	180	375	STEEL	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
12	181	282	375	STEEL	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

Length of Headpipe \_\_\_\_\_ Length of Tailpipe 5'

Packer ☐ Y ☒ N Type \_\_\_\_\_

## 9. PERFORATIONS/SCREENS PACKER TYPE

Perforation Method \_\_\_\_\_

Screen Type & Method of Installation JOHNSON WIRE WRAP

From	To	Slot Size	Number	Diameter	Material	Casing	Liner
282	352	.030		12	S.S.	<input checked="" type="checkbox"/>	<input type="checkbox"/>

## 10. FILTER PACK

Filter Material	From	To	Weight / Volume	Placement Method
#8-12 SAND	272	360	24,000	DRY POUR

## 11. STATIC WATER LEVEL OR ARTESIAN PRESSURE:

ft. below ground \_\_\_\_\_ Artesian pressure 6 lb.  
Depth flow encountered \_\_\_\_\_ ft. Describe access port or control devices:  
FLANGED CAP w/ 1 1/4" PIPE PLUG

## 12. WELL TESTS:

☒ Pump ☐ Baller ☐ Air ☐ Flowing Artesian

Yield gal/min.	Drawdown	Pumping Level	Time
1300 gpm	149'	135'	4 1/2 hrs

Water Temp. \_\_\_\_\_ Bottom hole temp. \_\_\_\_\_

Water Quality test or comments: \_\_\_\_\_

Depth first Water Encounter \_\_\_\_\_

## 13. LITHOLOGIC LOG: (Describe repairs or abandonment)

Bottom Dia.	From	To	Remarks: Lithology, Water Quality & Temperature	Water Y N
24	0	4	TOP SOIL	
4	4	7	CLAY	
7	7	11	SAND	
11	11	28	SAND, RIVER GRAVELS	
28	28	60	BEN CLAY	
60	60	63	SAND	
63	63	72	BEN CLAY	
72	72	92	SAND	
92	92	94	BEN CLAY	
94	94	166	SAND w/ BEN CLAY STREAKS	
166	166	174	CLAY	
174	174	178	SAND	
178	178	181	CLAY	
181	181	183	SAND, CLAY STREAKS	
183	183	207	SAND	
207	207	264	CLAY	
264	264	274	SAND w/ CLAY STREAKS	
274	274	353	SAND	
353	353	360	BLUE CLAY	
360	360	384	BLUE CLAY	
384	384	387	SAND	
387	387	419	BLUE - GRAY CLAY	
419	419	444	SAND	
444	444	455	BLUE - GRAY CLAY	
455	455	493	SAND w/ CLAY LAYERS	
493	493	501	BLUE - GRAY CLAY	
501	501	505	CEMENTED SAND	
505	505	513	BLUE - GRAY CLAY	

NOTE: BORE HOLE ABANDONMENT  
FROM 415' TO 513' WITH  
DRILL CUTTINGS

Completed Depth 357' (Measurable)  
Date: Started 1-24-06 Completed 4-17-06

## 14. DRILLER'S CERTIFICATION

I/We certify that all minimum well construction standards were complied with at the time the rig was removed.

Company Name RIVERSIDE INC Firm No. 333  
Principal Driller [Signature] Date 5-15-06  
and  
Driller or Operator II [Signature] Date \_\_\_\_\_  
Operator I [Signature] Date 5-15-06

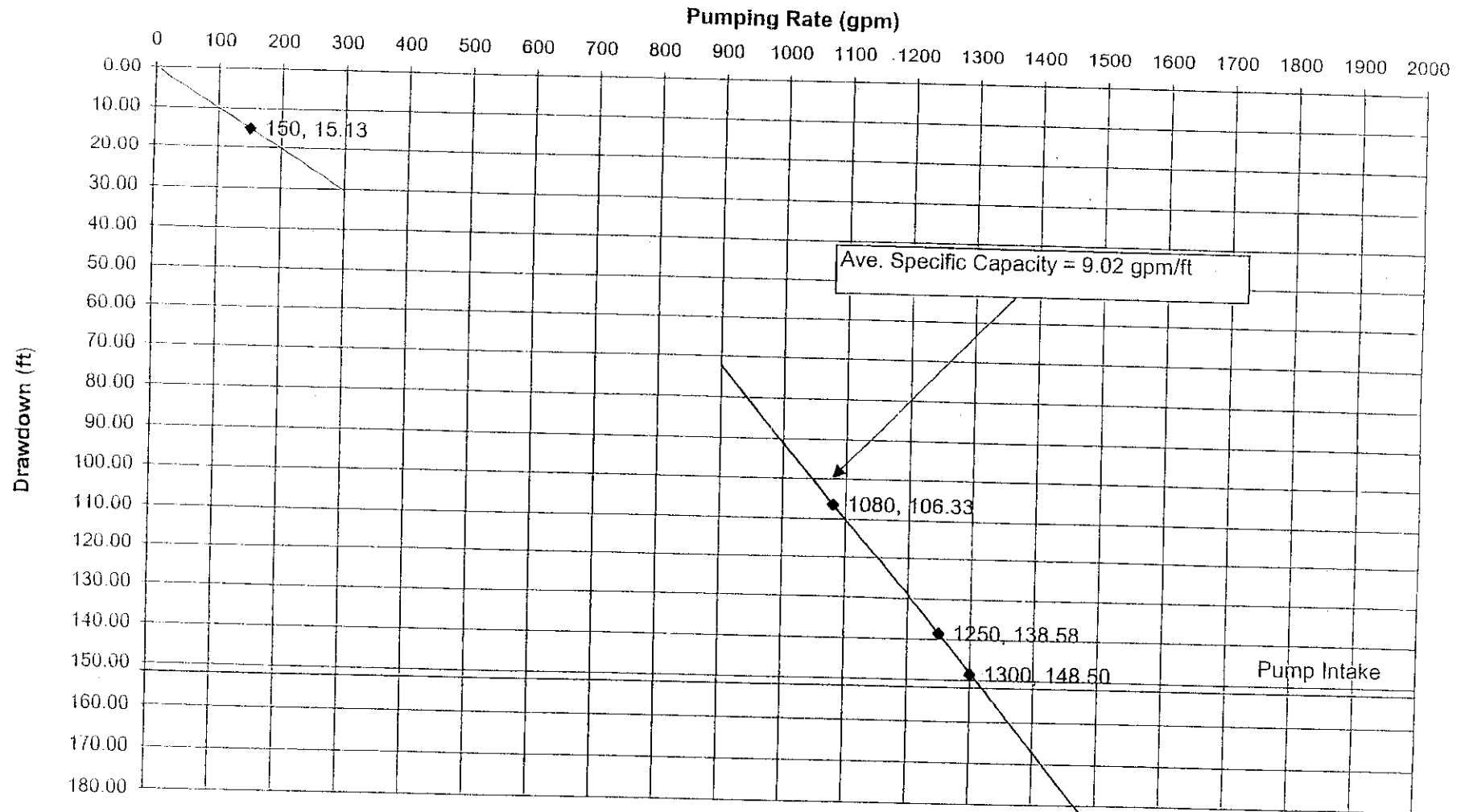
Principal Driller and Rig Operator Required.  
Operator I must have signature of Driller/Operator II.

FORWARD WHITE COPY TO WATER RESOURCES

TEST 11/11/06  
11/2.1

TEST WELL #1

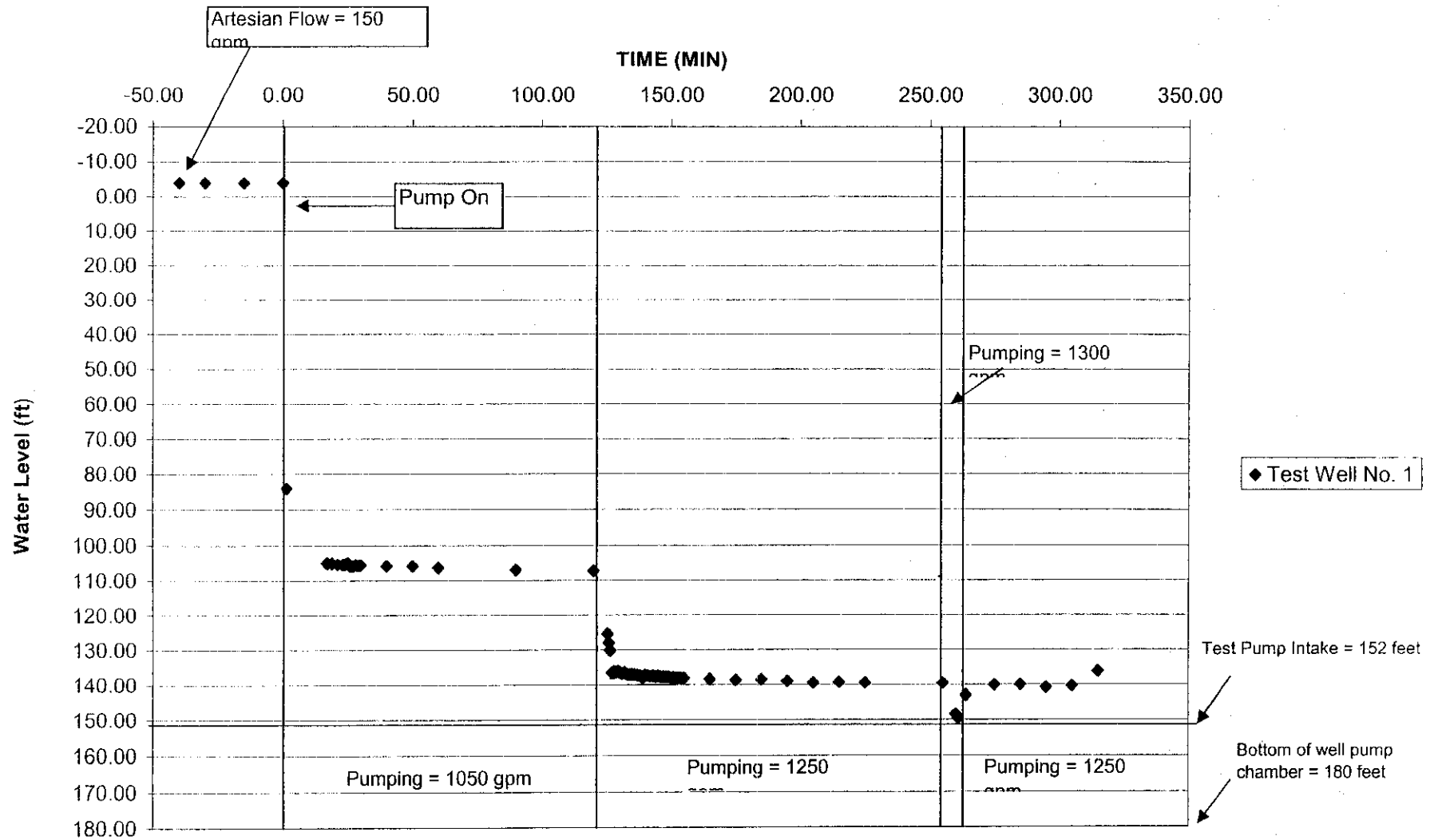
**STEP PUMP TEST of TEST WELL No.1 (Legacy)**  
**March 24th 9:15am to 2:30 pm**  
**Specific Capacity**

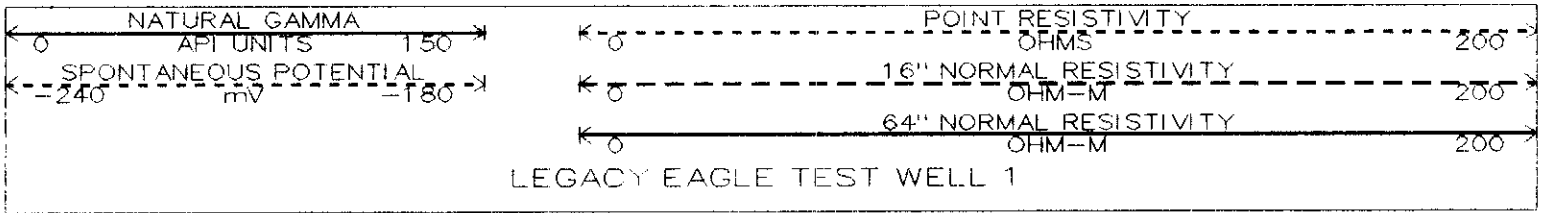
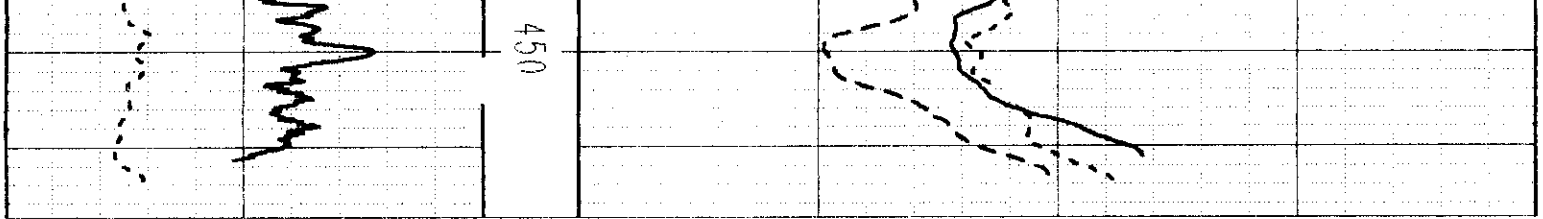


# STEP PUMP TEST of TEST WELL No.1 (Legacy)

March 24th 9:15am to 2:30 pm

## WATER LEVEL





COMPANY RIVERSIDE, INC.

DRILLER T.D. 465'

WELL LEGACY EAGLE TEST WELL 1

LOG F.R. 464'

AREA CITY OF EAGLE

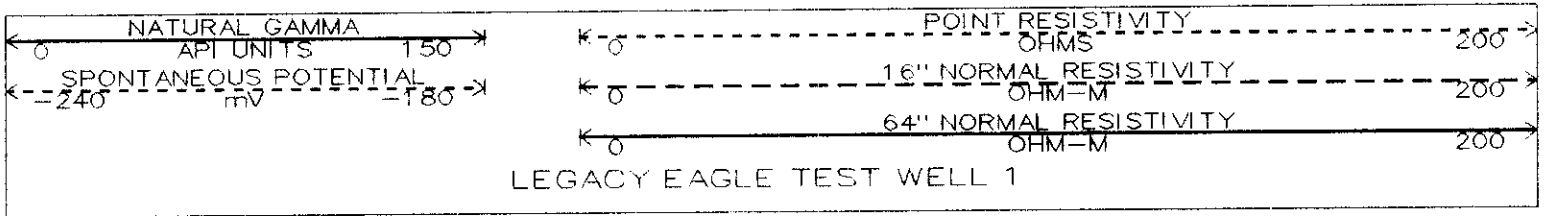
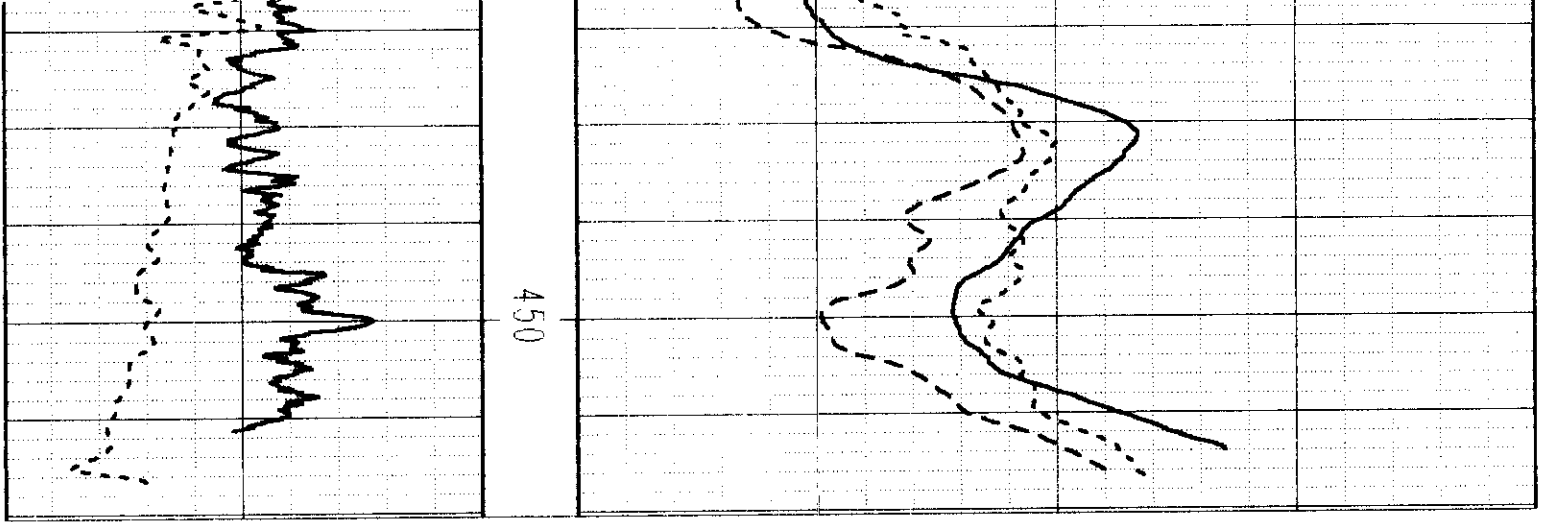
LOG T.D. 465'

COUNTY ADA

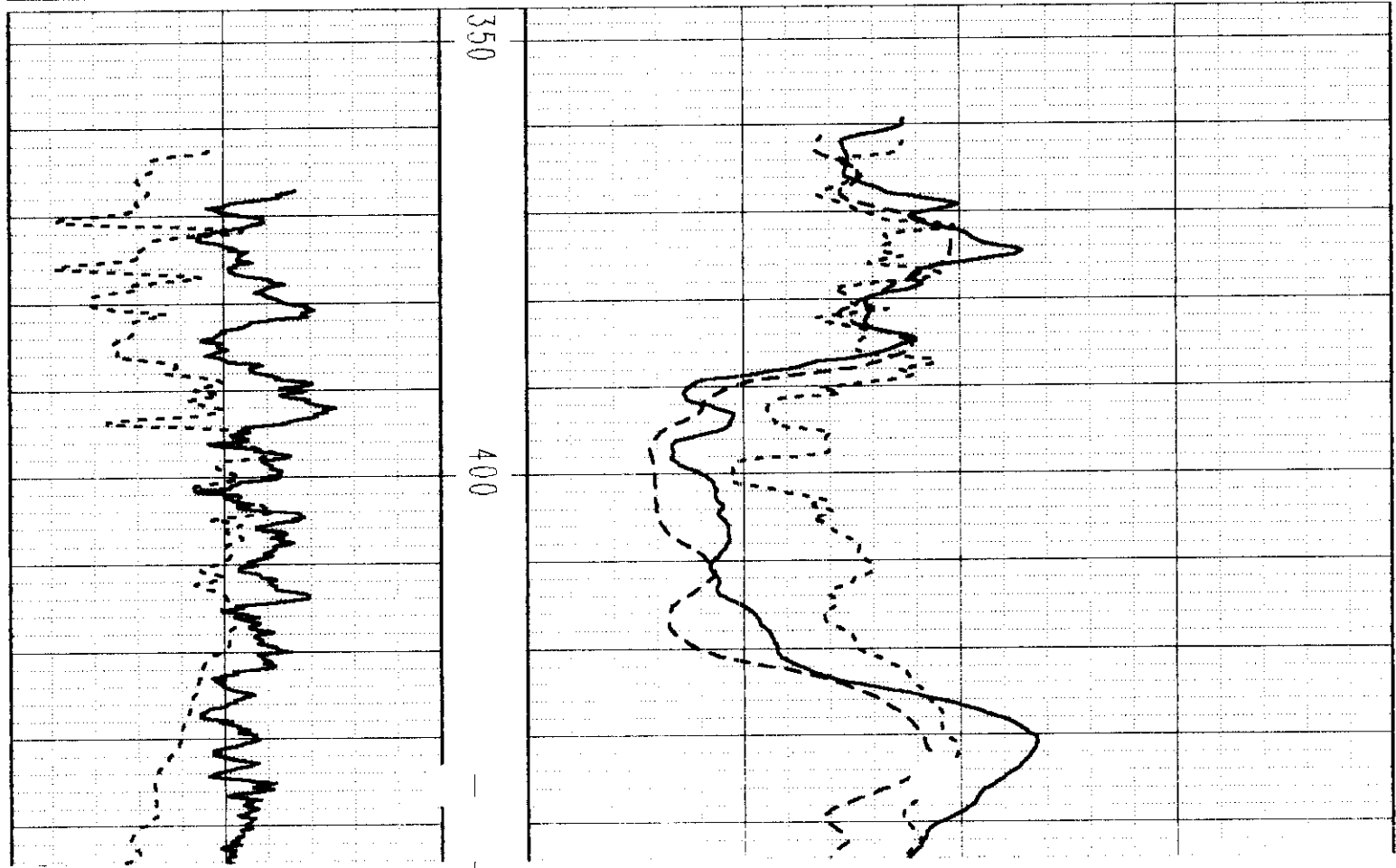
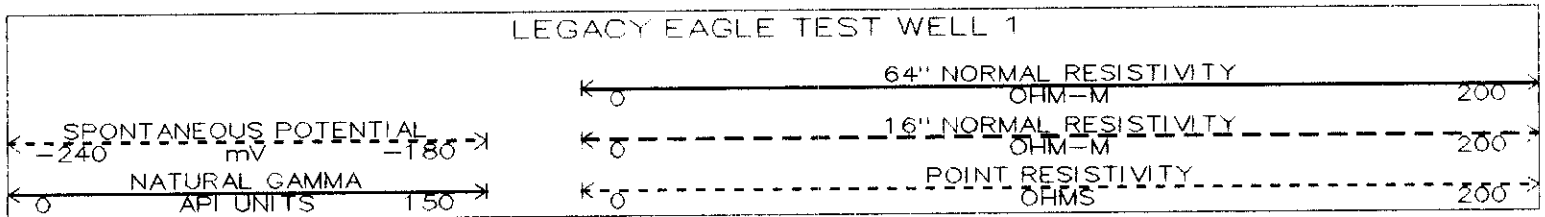
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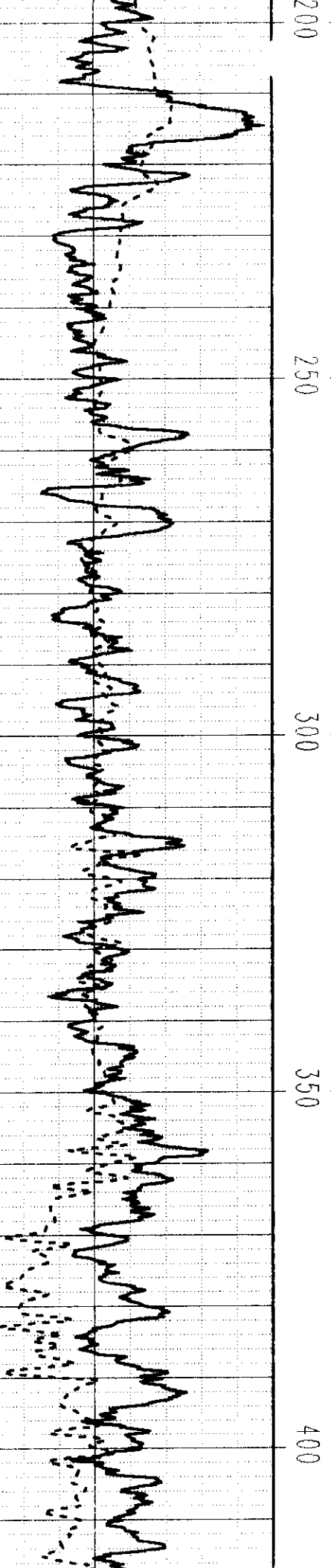
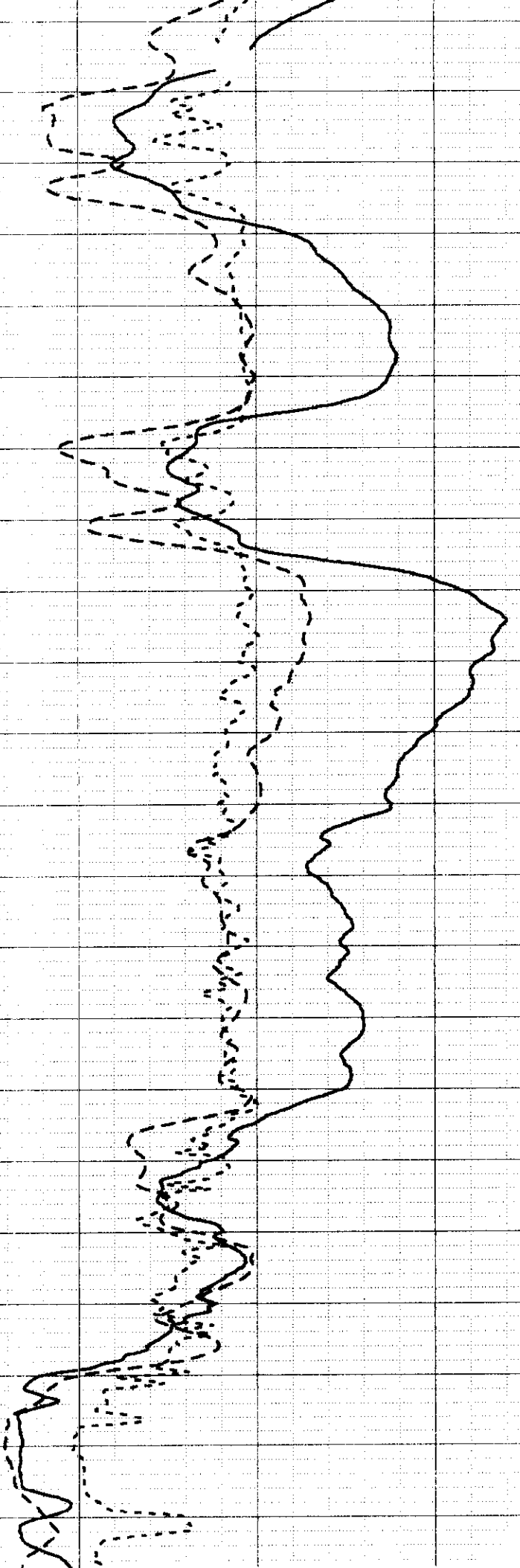
STATE IDAHO

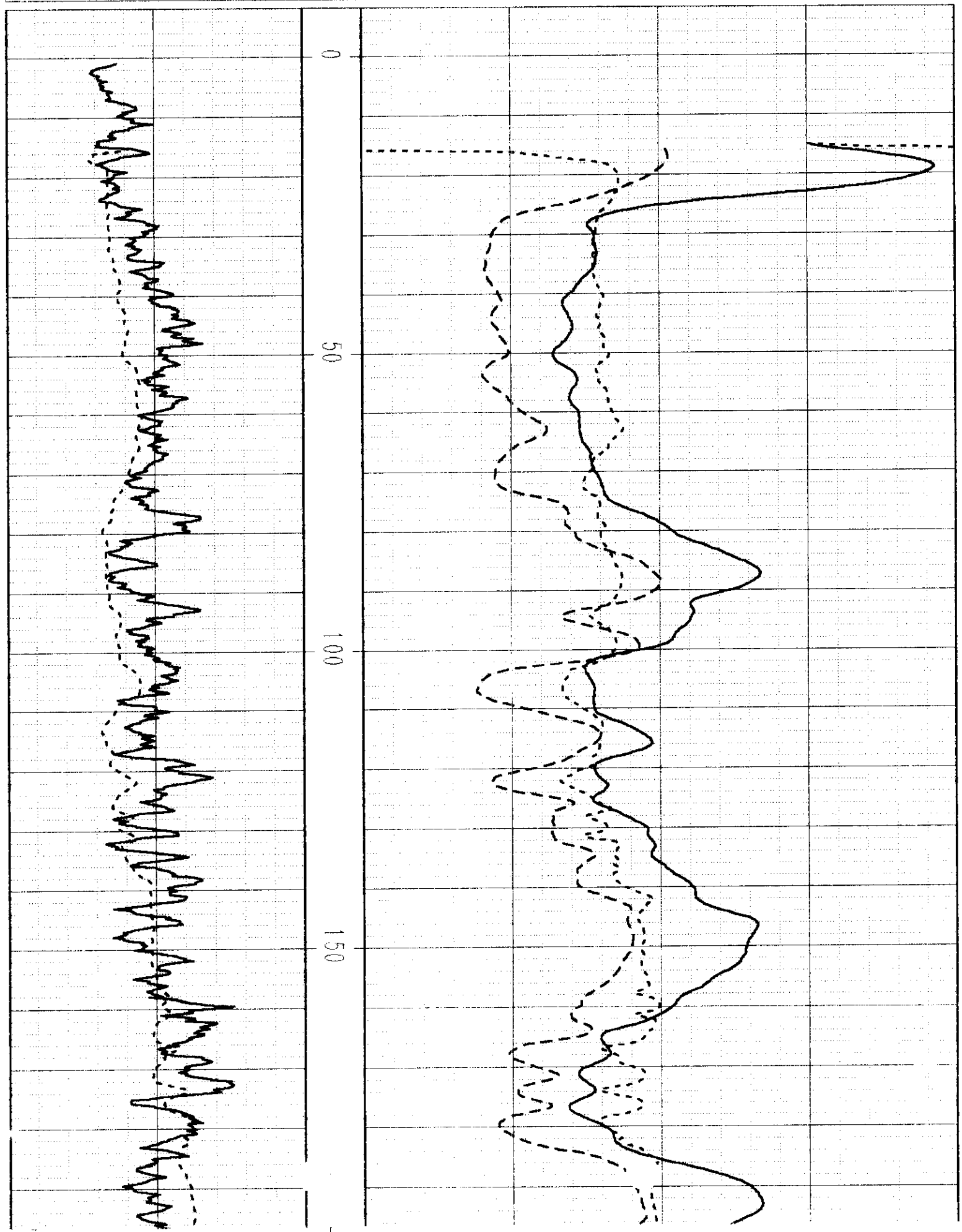
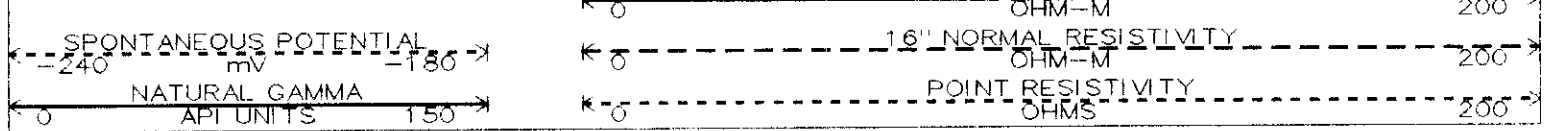
G.L. 2520' T.O.C.



## REPEAT SECTION







INC.

NATURAL GAMMA/SP  
64 INCH NORMAL RESISTIVITY  
16 INCH NORMAL RESISTIVITY  
POINT RESISTIVITY LOG

COMPANY: RIVERSIDE, INC.

WELL: LEGACY EAGLE TEST WELL 1

FIELD: CITY OF EAGLE

COUNTY: ADA STATE: IDAHO

OTHER LOGS:

NONE

NONE

PERMANENT DATUM: GROUND LEVEL

ELEVATION: 2520'est.

LOG MEASURED FROM: GROUND LEVEL

RUN NO.	ONE
---------	-----

DATE	02/03/06
------	----------

DRILLER: RIVERSIDE, INC.

BIT SIZE @ DEPTH	20' 8' to 465'
------------------	----------------

BIT SIZE @ DEPTH	
------------------	--

CASING SIZE @ DEPTH	32" 8'
---------------------	--------

[illegible]

LOGGER :

DEPTH	465'
-------	------

CASING DEPTH	8'
--------------	----

BOT LOG INTERVAL	755'
------------------	------

TOP LOG INTERVAL.	0'
-------------------	----

TYPE FLUID IN HOLE	MUD
--------------------	-----

Rm @ TEMP	N/A
-----------	-----

SAMPLE SOURCE	N/A
---------------	-----

FLUID LEVEL	FULL
-------------	------

TIME SINCE CIRC.	N/A
------------------	-----

RECORDED BY:	Oneyear
--------------	---------

WITNESSED BY:	Mr. Duncan
---------------	------------

## LOGGING DATA

[illegible]

CALIBRATION FACTOR(S):Downhole and shop calibration.

DIGITAL FILE NAME(S):1.LOG

REMARKS:

MSI Acquire Type 3

Gamma calibrated with 120 API Standard.

LEGACY EAGLE TEST WELL 1

54" NORMAL RESISTIVITY



# Analytical Laboratories, Inc.

1804 N. 33rd Street  
Boise, Idaho 83703  
Phone (208) 342-5515

Date Report Printed: 4/4/2006 8:40:48  
<http://www.analyticallaboratories.com>

## Laboratory Analysis Report

Sample Number: 0608752

Attn: CHRIS DUNCAN  
HOLLADAY ENGINEERING CO  
32 N MAIN  
PO BOX 235  
PAYETTE, ID 83661

Collected By: C DUNCAN  
Submitted By: B TOMISSER

Source of Sample:  
TEST WELL #1 (GROUNDWATER)

Time of Collection: 10:30  
Date of Collection: 3/24/2006  
Date Received: 3/24/2006  
Report Date: 4/4/2006

PWS#:

PWS Name:

Test Requested	MCL	Analysis Result	Units	MDL	Method	Date Completed	Analyst
Arsenic Low	0.01	<0.005	mg/L	0.005	EPA 200.8	3/31/2006	JH
Iron, Fe	UR	0.08	mg/L	0.05	EPA 200.7	3/27/2006	KC
Magnesium, Mg	UR	4.97	mg/L	0.10	EPA 200.7	3/29/2006	KC
Manganese, Mn	UR	<0.05	mg/L	0.05	EPA 200.7	3/27/2006	KC
Nitrate (as N)	10	<0.2	mg/L	0.2	EPA 300.0	3/25/2006	BF
Nitrite (as N)	1.00	<0.01	mg/L	0.01	EPA 353.2	3/25/2006	BF
Fluoride, F	4.0	0.45	mg/L	0.1	EPA 300.0	3/28/2006	WW
Sulfate, SO4	UR	14	mg/L	1	EPA 300.0	3/28/2006	WW

*Michael Moore* 4/4/06

Thank you for choosing Analytical Laboratories for your testing needs.

If you have any questions about this report, or any future analytical needs, please contact: Michael Moore

MCL = Maximum Contamination Level  
MDL = Method/Minimum Detection Limit  
UR = Unregulated



# Analytical Laboratories, Inc.

1804 N. 33rd Street  
Boise, Idaho 83703  
Phone (208) 342-5515

Date Report Printed: 9/12/2006 10:55:4  
<http://www.analyticallaboratories.com>

## Laboratory Analysis Report

Sample Number: 0608753

**Attn:** CHRIS DUNCAN  
HOLLADAY ENGINEERING CO  
32 N MAIN  
PO BOX 235  
PAYETTE, ID 83661

**Collected By:** C DUNCAN  
**Submitted By:** B TOMISSER

**Source of Sample:**  
TEST WELL #1 (GROUNDWATER)

**Time of Collection:** 10:30  
**Date of Collection:** 3/24/2006  
**Date Received:** 3/24/2006  
**Report Date:** 9/7/2006

**PWS#:**

**PWS Name:**

Test Requested	MCL	Analysis Result	Units	MDL	Method	Date Completed	Analyst
Gross Alpha		4.6+/-2.8	pCi/L		EPA 900.0	8/21/2006	IBOL
Gross Alpha testing by Idaho Bureau of Laboratories (IBOL).							
Gross Beta		3.9	pCi/L		EPA 900.0	8/21/2006	IBOL
Gross Beta testing by Idaho Bureau of Laboratories (IBOL).							

*Michael Moore* 9/13/06

Thank you for choosing Analytical Laboratories for your testing needs.

MCL = Maximum Contamination Level  
MDL = Method/Minimum Detection Limit  
UR = Unregulated

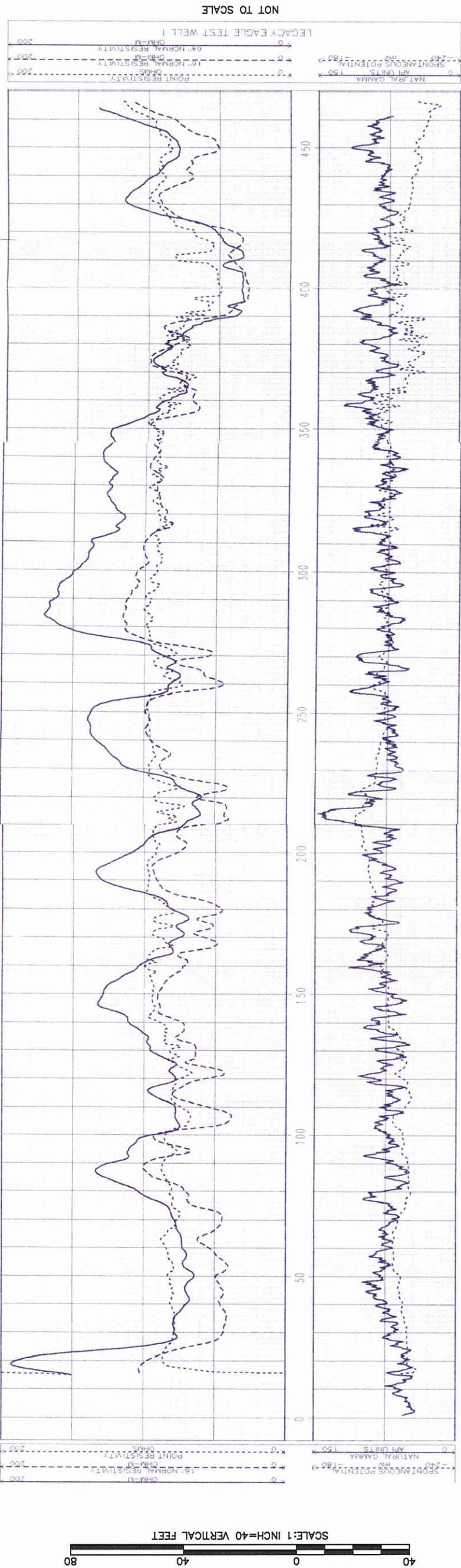
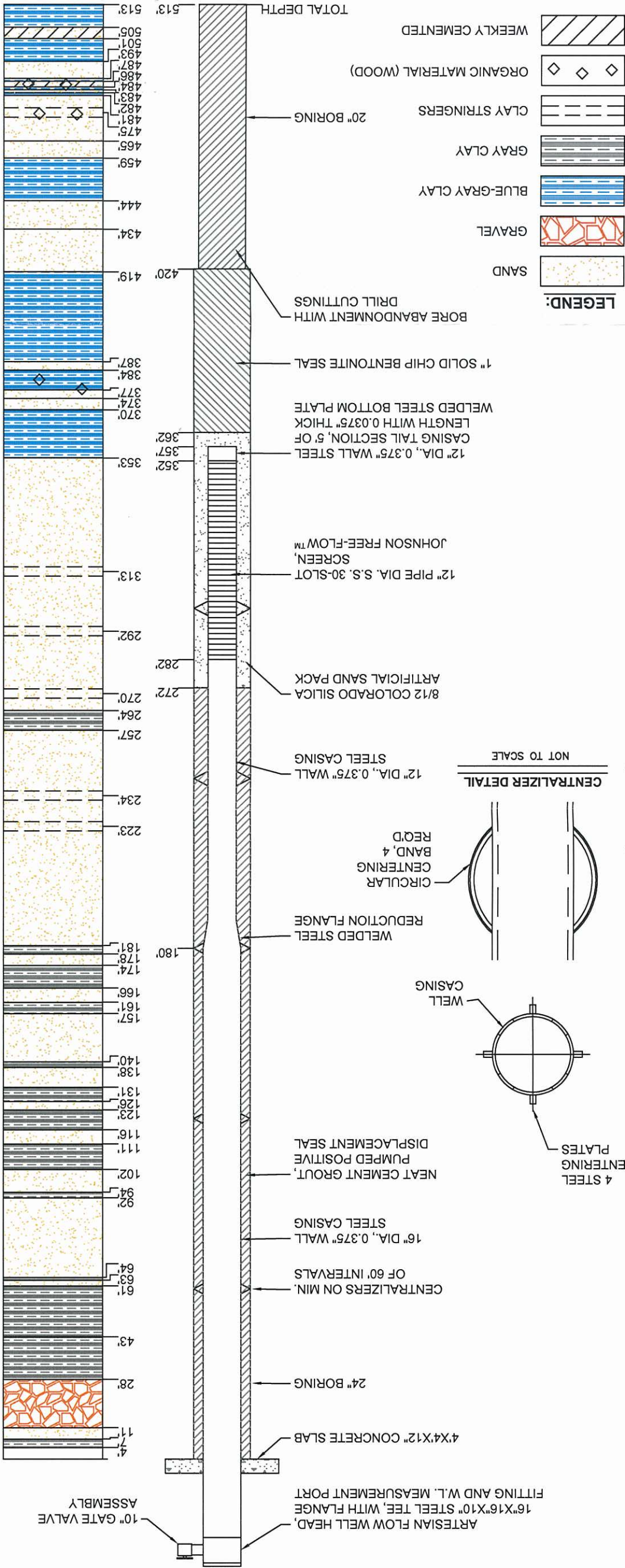
If you have any questions about this report, or any future analytical needs, please contact: **Michael Moore**

AS-BUILT 2-17-06  
EAGLE TEST WELL NO. 1  
CITY OF EAGLE, IDAHO

HOLLADAY ENGINEERING CO.  
ENGINEERS • CONSULTANTS  
32 N. MAIN P.O. BOX 235 PAYETTE, ID 83661  
PHONE: (208) 642-3304 • FAX: (208) 642-2159  
EMAIL: hce@holladayengineering.com

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OR BY ANY MEANS, ELECTRONIC  
OR MECHANICAL, INCLUDING  
PHOTOCOPYING, RECORDING,  
OR BY ANY INFORMATION  
SYSTEMS WITHOUT THE  
WRITTEN AUTHORIZATION OF  
HOLLADAY ENGINEERING CO.

JOB NO. EG 061204  
CADD FILE: C:\WORK\AS-BUILT\WELL TEST\WELL  
DATE: 2/10/06  
REVISED: 3/13/06  
PLOTTER: 8/30/06  
DRAWN BY: MCM  
CHECKED BY: CHD  
SHEET



## WELL DRILLER'S REPORT

Office Use Only

Inspected by

Twp Age Sec

1/4 1/4 1/4

Lat: Long:

1. WELL TAG NO. D 0042405  
DRILLING PERMIT NO. 832605 - 837870  
Water Right or Injection Well No. 63-32039, 63-32090

## 2. OWNER:

Name CITY OF EAGLE  
Address 310 E STATE ST  
City EAGLE State IA Zip 52521

## 3. LOCATION OF WELL by legal description:

You must provide address or Lot, Blk, Sub. or Directions to well. WELL #2

Twp. 4 North ☒ or South 1 (EAGLEFIELD)  
Rge. 1 East ☐ or West ☒  
Sec. 11 1/4 NW 1/4 SE 1/4  
Gov't Lot ADA

Lat: Long:  
Address of Well Site W. OF INTERSECTION OF WATLOCK DR +  
GOLDEN GARDEN DR City EAGLE  
US 84 Blk. 8 Sub. Name EAGLEFIELD ESTATES

## 4. USE:

☐ Domestic ☒ Municipal ☐ Monitor ☐ Irrigation  
☐ Thermal ☐ Injection ☒ Other TEST

## 5. TYPE OF WORK check all that apply

(Replacement etc.)

☒ New Well ☐ Modify ☐ Abandonment ☐ Other

## 6. DRILL METHOD:

☐ Air Rotary ☐ Cable ☐ Mud Rotary ☒ Other REVERSE

## 7. SEALING PROCEDURES

Seal Material	From	To	Weight / Volume	Seal Placement Method
CEMENT GROUT	0	320	25 LBS	PUMPED FROM BOTTOM TO TOP

Was drive shoe used? ☐ Y ☒ N Shoe Depth(s)Was drive shoe seal tested? ☐ Y ☐ N How?8. CASING/LINER: 280' 16" X 12" REVERSE

Diameter	From	To	Gauge	Material	Casing	Liner	Welded	Threaded
16	± 5	280	37.5	STEEL	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
18	281	345	38	STEEL	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

Length of Headpipe Length of Tailpipe 5'Packer ☐ Y ☒ N Type

## 9. PERFORATIONS/SCREENS PACKER TYPE

Perforation Method

Screen Type & Method of Installation JOHNSON WIRE WARP

From	To	Slot Size	Number	Diameter	Material	Casing	Liner
245	425	230		12	S.S.	<input checked="" type="checkbox"/>	<input type="checkbox"/>

## 10. FILTER PACK

Filter Material	From	To	Weight / Volume	Placement Method
#10-20 SS SAND	320	425	30,000	DRY POUR

## 11. STATIC WATER LEVEL OR ARTESIAN PRESSURE:

ft. below ground Artesian pressure 8.1 lb.

Depth flow encountered ft. Describe access port or control devices:

FLANGE CAP W/ 2 1/4" PIPE PLUGS

## 12. WELL TESTS:

☐ Pump ☐ Baller ☐ Air ☐ Flowing Artesian

Yield gal./min.	Drawdown	Pumping Level	Time
1123 gpm		15.2'	10 min

Water Temp. Bottom hole temp.

Water Quality test or comments:

Depth first Water Encounter

## 13. LITHOLOGIC LOG: (Describe repairs or abandonment)

Bore Dia.	From	To	Remarks: Lithology, Water Quality & Temperature	Water	Y	N
28	0	3	TOP SOIL			
	3	28	RIVER GRAVEL			
	28	48	TAN CLAY			
	48	56	SAND			
	56	63	RUSTY TAN CLAY			
	63	135	SAND w/SM TAN CLAY STRAINS			
	135	183	BAN CLAY w/SAND STRAINS			
	183	189	FINE-MED SAND			
	189	194	TAN CLAY			
	194	215	SAND SH. REVEAL			
	215	232	CLAY			
	232	290	SAND FINE-MED w/CLAY STRAINS			
	290	297	SAND-FINE			
	297	339	BR. RUSTY, GREEN-GRAY CLAY			
	339	436	COARSE SAND, SOME FINE GRAIN			
	436	444	BAN RUSTY CLAY			

Completed Depth 430' (Measurable)Date: Started 2-21-06 Completed 5-8-06

## 14. DRILLER'S CERTIFICATION

I/We certify that all minimum well construction standards were complied with at the time the rig was removed.

Company Name DIVERSIDE INC Firm No. 333Principal Driller [Signature] Date 5-12-06and Driller or Operator II Tyler Chesser Date 5-22-06

Operator I \_\_\_\_\_ Date \_\_\_\_\_

Principal Driller and Rig Operator Required.  
Operator I must have signature of Driller/Operator II.

FORWARD WHITE COPY TO WATER RESOURCES

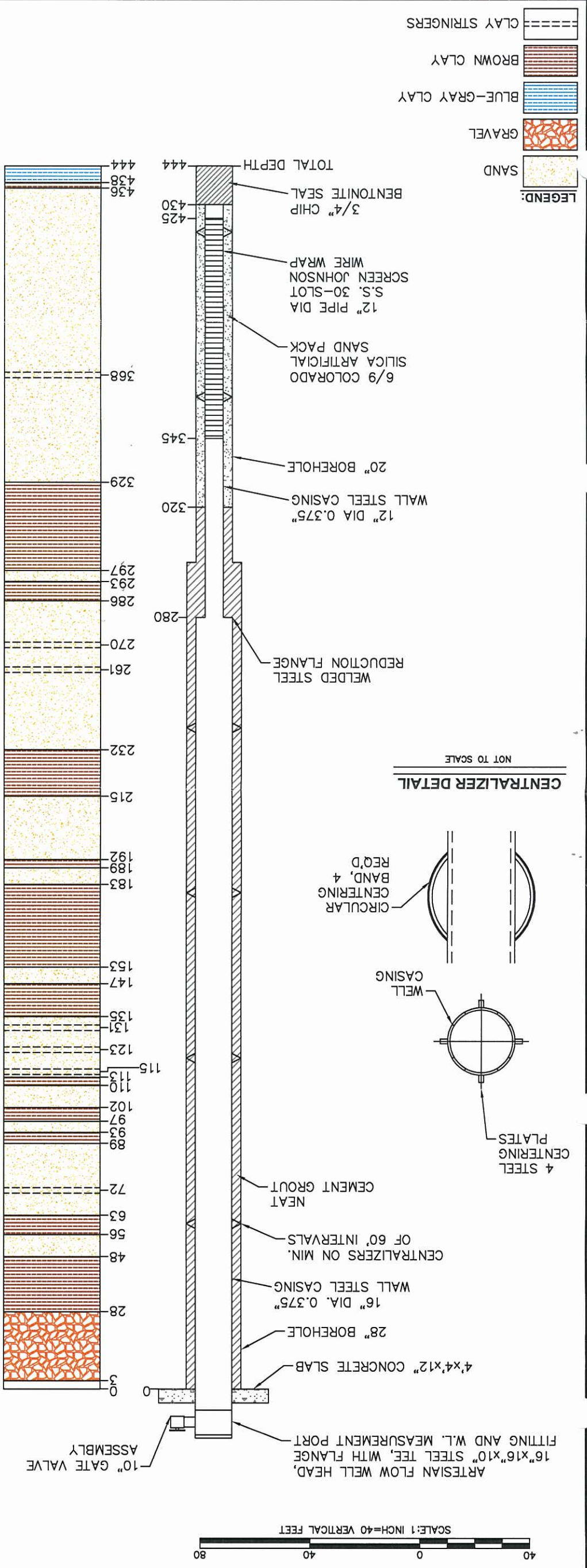
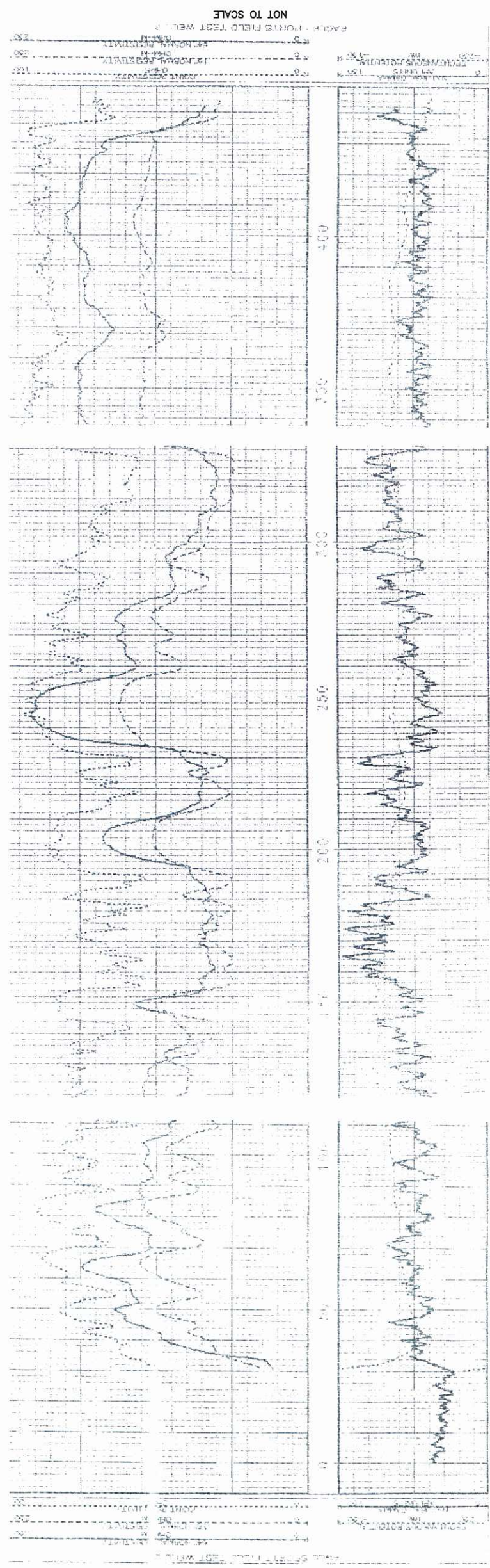
TEST WELL

NO. 2

TEST WELL #2



NOTE: WELL CONSTRUCTION BY SPF WATER ENGINEERING





April 8, 2006

Peter Harris  
Eaglefield, LLC  
6951 Duncan Lane  
Boise, ID 83714

Subject: Final Design for Eaglefield Well No. 1 (City of Eagle Test Well 4)

Dear Peter:

Drilling of the municipal test well borehole at Eaglefield was completed to approximately 444 feet on April 7, 2006. The borehole is nominal 20-inch diameter and was advanced using the reverse rotary method. The drill cuttings consist primarily of brown sand, tan clay, and brown clay above 438 feet. Blue-gray clay was encountered from 438 to 444 feet.

Geophysical logging was conducted on April 7. Strata Data, Inc. from Casper, Wyoming, was the geophysical contractor. Logs run include natural gamma radiation, spontaneous potential, single point resistivity, 16-inch normal resistivity, and 64-inch normal resistivity.

Drill cuttings and geophysical logs document the presence of a potentially productive sand layer from 330 feet to 433 feet. This sand layer corresponds with the target interval for this well, and we propose completing the well with screens extending from 345 to 425 feet.

The completed well will include 16-inch casing from approximately 5 feet above ground surface to 280 feet, 12-inch casing from 280 feet to 345 feet, 12-inch well screen from 345 feet to 425 feet, and 12-inch tail pipe with plate bottom from 425 feet to 430 feet. A 16-inch by 12-inch welded reducer will be used to connect the 12-inch and 16-inch casing strings. The invert of the 10-inch artesian by-pass line will be approximately 2.5 feet above existing ground surface.

No. 6-9 Colorado Silica Sand filter pack will be installed in the borehole annulus from 320 to 430 feet. Estimated volume is approximately 5 cubic yards.

Bentonite chips will be used as surface seal material. The chips will extend from 320 feet to ground surface. Estimated volume is approximately 34 cubic yards. Prior to installation of casing and screen, the borehole from ground surface to 300 feet will be reamed to 28-inch diameter to facilitate installation of the bentonite surface seal.

We are still discussing well development options with Riverside. Our current thinking is to develop the well initially by bailing, air lifting, or pumping until artesian flow is

April 8, 2006

achieved. After the initial artesian flow clears, development will continue by shutting in the well, pressurizing the casing with compressed air, and then releasing pressure to allow artesian flow. This procedure can be repeated until the artesian flow clears. Air pressures should start low, and gradually increase up to 100 psi. The artesian flow rate and shut-in pressure following initial development will allow a preliminary determination of well capacity as soon as possible. Final development will be completed using the test pump.

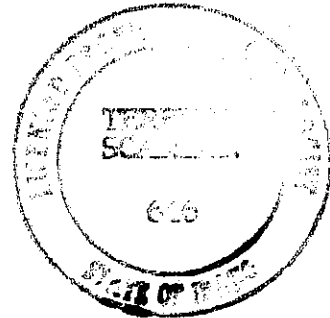
A well design schematic, drill log, and field geophysical logs are enclosed for your records.

Please contact me with any questions.

Sincerely,



Terry M. Scanlan, P.E., P.G.

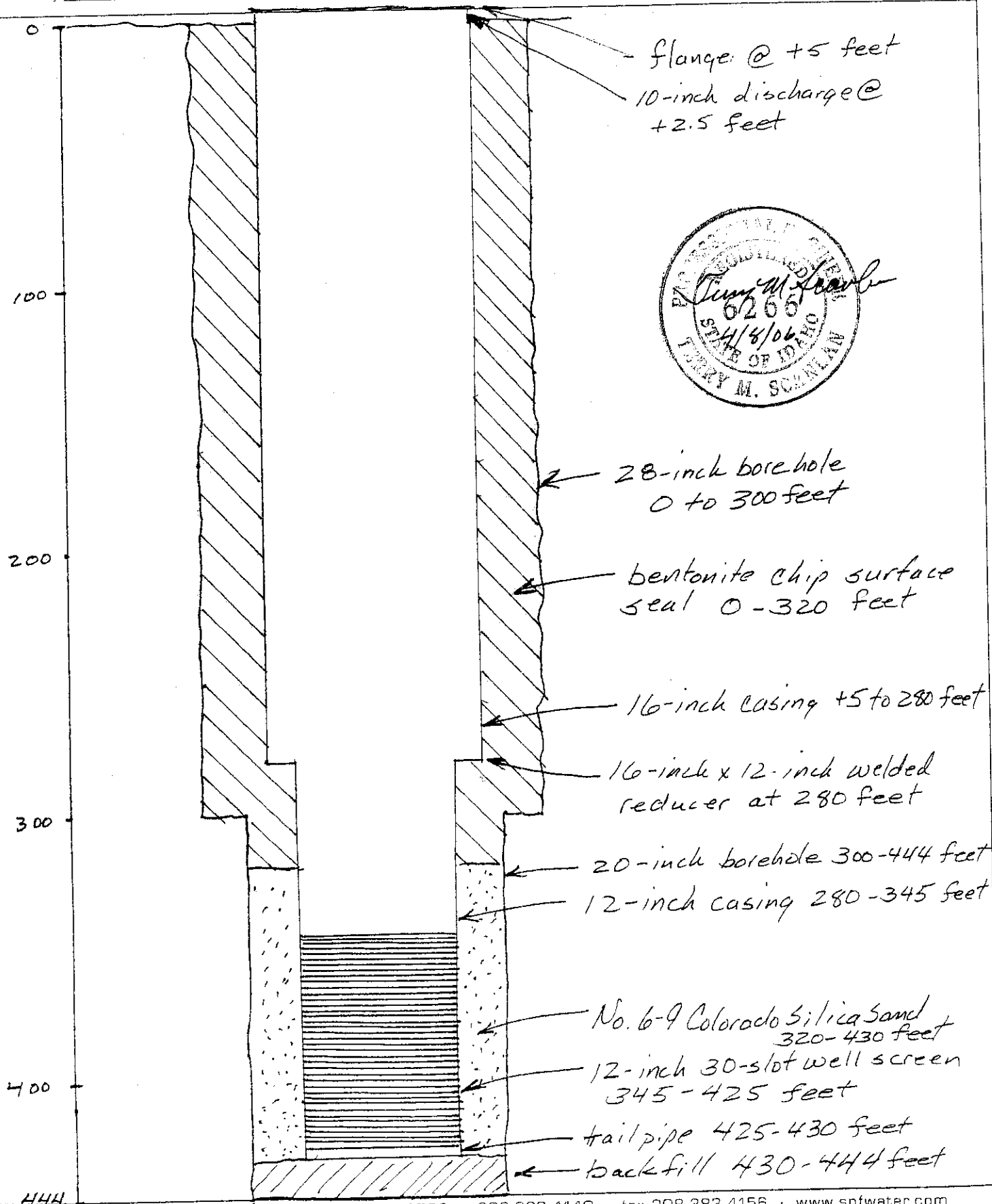


cc: Terry Daugherty – Riverside, Inc.  
Rob Whitney – Idaho Department of Water Resources  
Chris Duncan – Holladay Engineering

Attachments: Well Schematic  
Drill Log  
Geophysical Logs

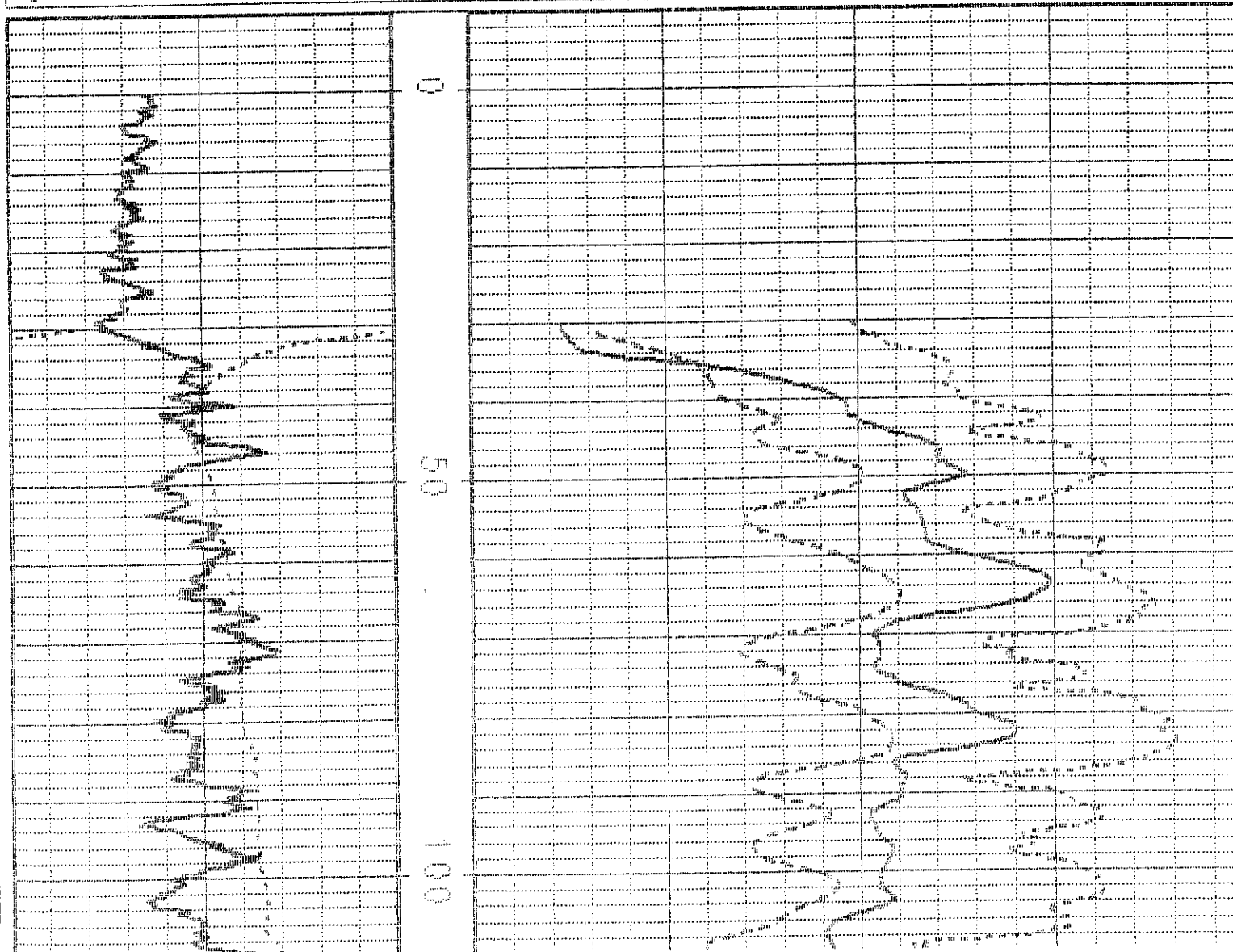


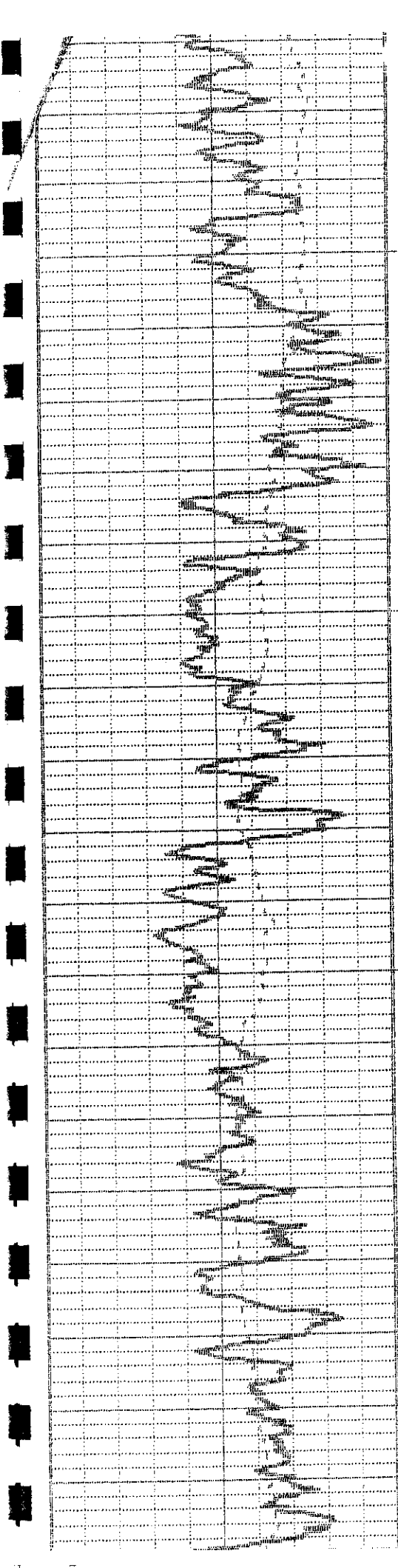
By TS Date 4/4/06 Client Eaglefield Sheet 1 of 1  
Chkd By \_\_\_\_\_ Description Final Well Design - Eagle Well #4 Job No 421.0010



# EAGLE SPORTS FIELD TEST WELL 2

SPONTANEOUS POTENTIAL MV	54" NORMAL RESISTIVITY OHM-M
NATURAL GAMMA API UNITS	16" NORMAL RESISTIVITY OHM-M
	POINT RESISTIVITY OHMS



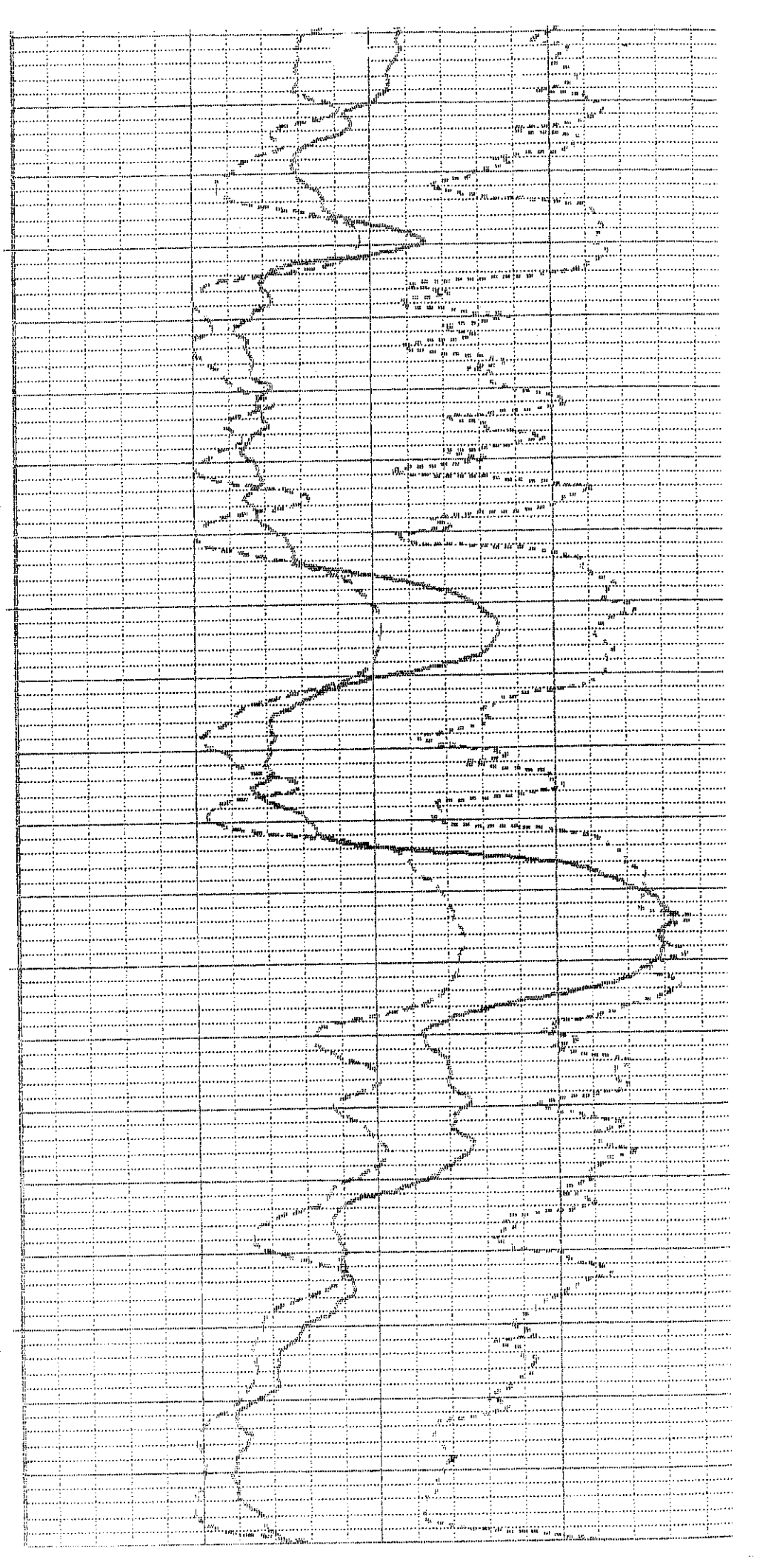


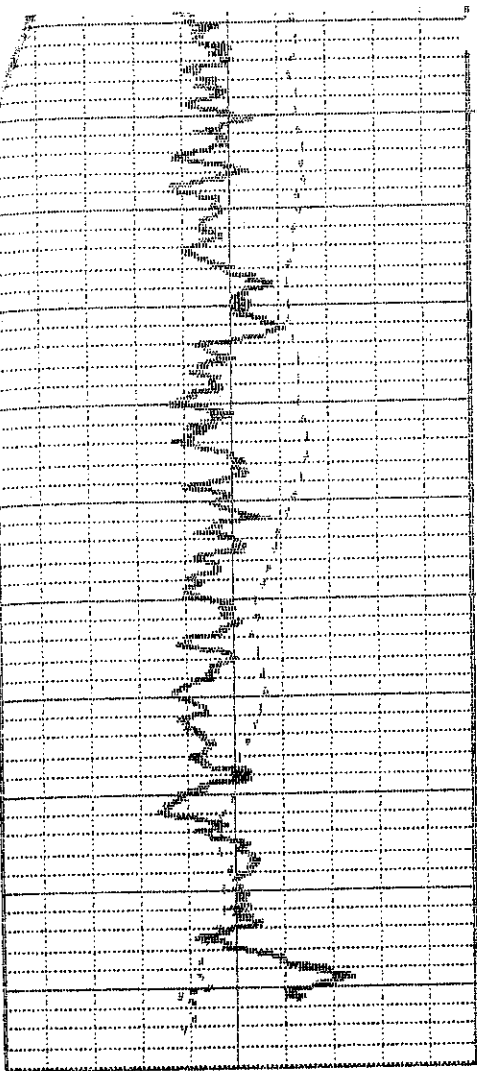
150

200

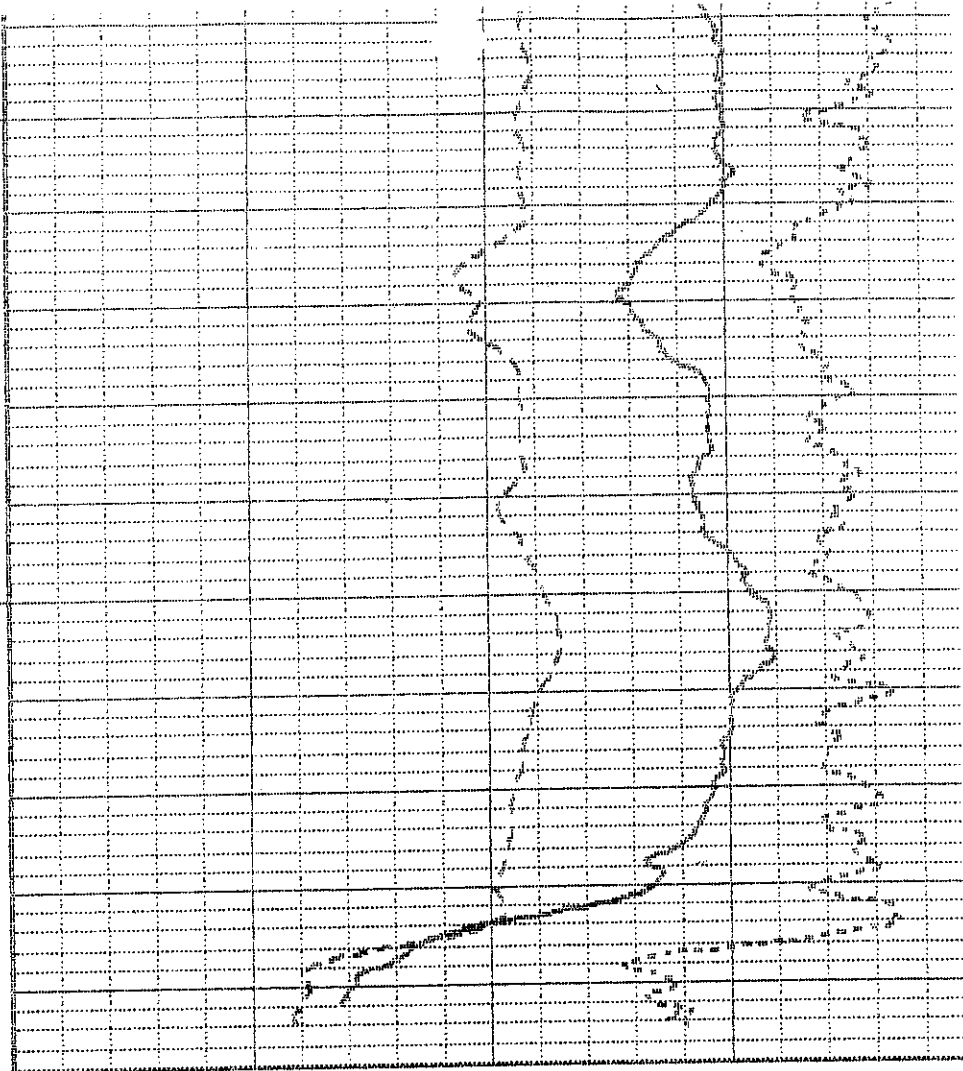
250

300





350  
400



NATURAL GAMMA  
API UNITS 150  
SPONTANEOUS POTENTIAL  
mv -250 -150

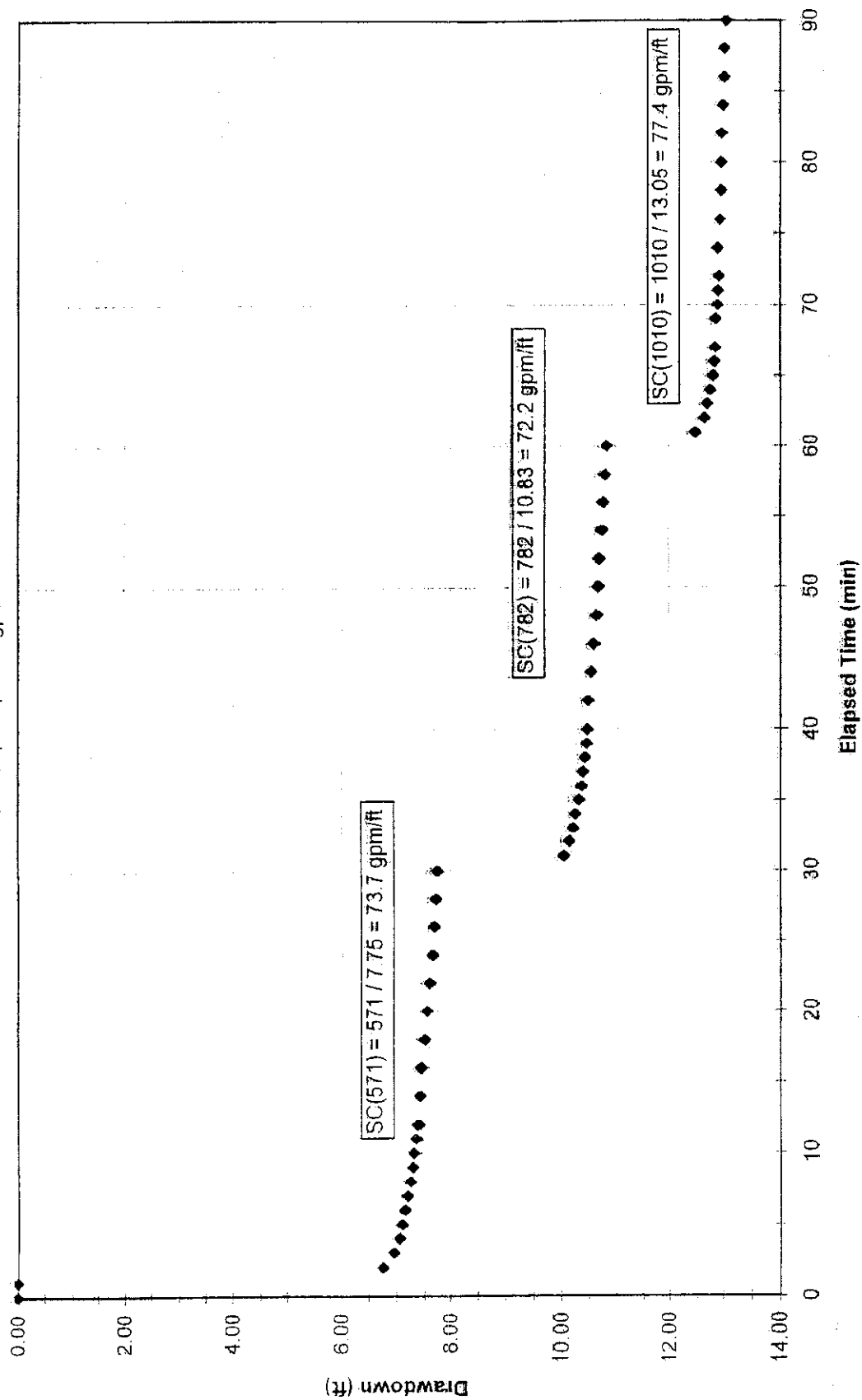
POINT RESISTIVITY  
OHMS 100  
16" NORMAL RESISTIVITY  
OHM-M 250  
64" NORMAL RESISTIVITY  
OHM-M 250

EAGLE SPORTS FIELD TEST WELL 2

# Eaglefield Well No. 1 Step Test

Test Date: 5/16/06

Q = 571, 782, 1010 gpm



Eaglefield Well No. 1 Step Test, Q = 571, 782, 1010 gpm					
Test conducted by: SPF Water					
Flow measured by: 10x7 orifice, h = 8, 15, 25-inches					
Water level measured by: manometer, all water levels above ground surface					
Date	Time	WL (ft)	Elapsed Time (min)	DD (ft)	Remarks
5/16	12:56	18.75			static
5/16	13:00		0	0.00	open gate valve
5/16	13:01	18.75	1	0.00	adjust h=8"
5/16	13:02	12.00	2	6.75	
5/16	13:03	11.80	3	6.95	
5/16	13:04	11.70	4	7.05	
5/16	13:05	11.65	5	7.10	
5/16	13:06	11.60	6	7.15	
5/16	13:07	11.55	7	7.20	
5/16	13:08	11.50	8	7.25	
5/16	13:09	11.45	9	7.30	
5/16	13:10	11.43	10	7.32	
5/16	13:11	11.39	11	7.36	
5/16	13:12	11.35	12	7.40	T=16.3, pH=7.44, EC/SC=208.1/244.0
5/16	13:14	11.32	14	7.43	
5/16	13:16	11.30	16	7.45	
5/16	13:18	11.23	18	7.52	T=15.9, pH=7.46, EC/SC=209.9/246.4
5/16	13:20	11.19	20	7.56	
5/16	13:22	11.14	22	7.61	
5/16	13:24	11.09	24	7.66	
5/16	13:26	11.06	26	7.69	
5/16	13:28	11.03	28	7.72	T=16.1, pH=7.45, EC/SC=209.9/246.4
5/16	13:30	11.00	30	7.75	increase h=15"
5/16	13:31	8.70	31	10.05	
5/16	13:32	8.60	32	10.15	
5/16	13:33	8.53	33	10.22	
5/16	13:34	8.49	34	10.26	
5/16	13:35	8.42	35	10.33	
5/16	13:36	8.38	36	10.37	
5/16	13:37	8.35	37	10.40	
5/16	13:38	8.31	38	10.44	
5/16	13:39	8.28	39	10.47	difficult to read manometer, behind duct
5/16	13:40	8.27	40	10.48	tape
5/16	13:42	8.25	42	10.50	
5/16	13:44	8.20	44	10.55	T=16.1, EC/SC=208.8/245.6
5/16	13:46	8.15	46	10.60	
5/16	13:48	8.10	48	10.65	
5/16	13:50	8.07	50	10.68	
5/16	13:52	8.05	52	10.70	
5/16	13:54	8.00	54	10.75	T=16.0, pH=7.38, EC/SC=208.2/246.4
5/16	13:56	7.98	56	10.77	
5/16	13:58	7.95	58	10.80	T=16.0, pH=7.37, EC/SC=208.0/244.9
5/16	14:00	7.92	60	10.83	
5/16	14:01	6.28	61	12.47	increase h=25"
5/16	14:02	6.11	62	12.64	

Date	Time	WL (ft)	Elapsed Time (min)	DD (ft)	Remarks
5/16	14:03	6.05	63	12.70	
5/16	14:04	6.00	64	12.75	
5/16	14:05	5.94	65	12.81	
5/16	14:06	5.92	66	12.83	
5/16	14:07	5.90	67	12.85	check on flow along discharge channel, mised reading adjust gate valve couple of turns
5/16	14:09	5.89	69	12.86	
5/16	14:10	5.85	70	12.90	
5/16	14:11	5.84	71	12.91	
5/16	14:12	5.83	72	12.92	
5/16	14:14	5.85	74	12.90	T=16.0, pH=7.43, EC/SC=208.6/245.6
5/16	14:16	5.81	76	12.94	
5/16	14:18	5.79	78	12.96	
5/16	14:20	5.79	80	12.96	T=16.0, pH=7.42, EC/SC=208.7/245.7
5/16	14:22	5.78	82	12.97	
5/16	14:24	5.75	84	13.00	
5/16	14:26	5.73	86	13.02	
5/16	14:28	5.73	88	13.02	
5/16	14:30	5.70	90	13.05	closed gate valve



# Analytical Laboratories, Inc.

1804 N. 33rd Street  
Boise, Idaho 83703  
Phone (208) 342-5515

<http://www.analyticallaboratories.com>

Date Report Printed: 2/2006 5:21:05 PM

Attn: TERRY SCANLAN, P.E., P.G.  
S P F WATER ENGINEERING, LLC  
600 E RIVER PARK LN STE 105  
BOISE, ID 83706

Collected By: M MARTIN

Submitted By: M MARTIN

Source of Sample:  
EAGLE FIELD

Time of Collection: 14:45  
Date of Collection: 5/16/2006  
Date Received: 5/16/2006  
Report Date: 6/2/2006

PWS:

## Laboratory Analysis Report

Sample Number: 0614882

Test Requested	MCL	Analysis Result	Units	MDL	Method	Date Completed	Analyst
Aluminum, Al		0.24	mg/L	0.10	EPA 200.7	5/18/2006	KC
Antimony Low		< 0.005	mg/L	0.005	EPA 200.8	5/19/2006	JH
Arsenic Low		< 0.005	mg/L	0.005	EPA 200.8	5/19/2006	JH
Barium, Ba		0.07	mg/L	0.05	EPA 200.7	5/18/2006	KC
Beryllium Low		< 0.0005	mg/L	0.0005	EPA 200.8	5/19/2006	JH
Cadmium Low		< 0.0005	mg/L	0.0005	EPA 200.8	5/19/2006	JH
Calcium Hardness		74.3	mg/L	0.25	EPA 200.7	5/19/2006	KC
Calcium, Ca		29.8	mg/L	0.10	EPA 200.7	5/19/2006	KC
Chromium Low		< 0.002	mg/L	0.002	EPA 200.8	5/19/2006	JH
Copper, Cu		< 0.01	mg/L	0.01	EPA 200.7	5/18/2006	KC
Iron, Fe		< 0.05	mg/L	0.05	EPA 200.7	5/18/2006	KC
Magnesium, Mg		5.22	mg/L	0.10	EPA 200.7	5/19/2006	KC
Manganese, Mn		< 0.05	mg/L	0.05	EPA 200.7	5/18/2006	KC
Mercury, Hg		< 0.0002	mg/L	0.0002	EPA 245.1	5/26/2006	KC
Nickel, Ni		< 0.02	mg/L	0.02	EPA 200.7	5/18/2006	KC
Potassium, K		1.7	mg/L	0.5	EPA 200.7	5/19/2006	KC
Selenium Low		< 0.005	mg/L	0.005	EPA 200.8	5/19/2006	JH
Silica		30.4	mg/L	0.25	EPA 200.7	5/18/2006	KC
Silver, Ag		< 0.005	mg/L	0.005	EPA 272.1	6/1/2006	JH
Sodium, Na		16.3	mg/L	0.10	EPA 200.7	5/19/2006	KC

MCL - Maximum Contamination Level  
MDL - Method/Minimum Detection Limit  
UR - Unregulated

# Laboratory Analysis Report

Sample Number: 0614882

Test Requested	MCL	Analysis Result	Units	MDL	Method	Date Completed	Analyst
Thallium Low	<	0.001	mg/L	0.001	EPA 200.8	5/19/2006	JH
Uranium, U		2	ug/L	1	EPA 200.8	5/26/2006	JH
Zinc, Zn	<	0.005	mg/L	0.005	EPA 200.7	5/18/2006	KC
Corrosivity		-0.9			Langelier	6/2/2006	WW
Calculated at room temperature, 20 deg C. Moderately aggressive.							
Nitrate (as N)	<	0.2	mg/L	0.2	EPA 300.0	5/16/2006	JT
Ammonia Direct (as N)	<	0.04	mg/L	0.04	EPA 350.1	5/24/2006	JT
Nitrite (as N)	<	0.01	mg/L	0.01	EPA 353.2	5/16/2006	JT
Alkalinity		110	mg/L Ca		SM 2320B	5/22/2006	JS
Chloride, Cl		3	mg/L	1	EPA 300.0	5/16/2006	JT
Conductivity		316	umhos	2	SM 2510B	5/16/2006	KC
Cyanide, Total		<0.005	mg/L	0.005	EPA 335.4	5/23/2006	WW
Fluoride, F		0.40	mg/L	0.10	EPA 300.0	5/16/2006	JT
pH		7.1	S.U.		EPA 150.1	5/16/2006	JS
Sulfate, SO4		16	mg/L	1	EPA 300.0	5/16/2006	JT
Turbidity	<	0.5	NTU	0.5	EPA 180.1	5/16/2006	KC
Sand		1	mg/L	0.600	EPA 160.2	5/19/2006	DLR
Surfactants		<0.01	mg/L	.01	SM 5540	5/26/2006	MDM
Total Dissolved Solids		114	mg/L	25	SM 2540C	5/23/2006	DLR

MCL = Maximum Contamination Level  
MDL = Method/Minimum Detection Limit  
UR = Unregulated

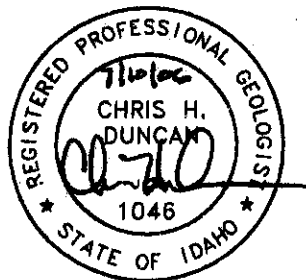
Michael Moore 6/5/2006

Thank you for choosing Analytical Laboratories for your testing needs.  
If you have any questions concerning this report,  
please contact: Michael Moore

# **CITY OF EAGLE, IDAHO**

## **7-Day Aquifer Test**

June 2006



EG061204

Prepared By

Holladay Engineering Company  
Payette, Idaho

# **CITY OF EAGLE, IDAHO**

## **7-Day Aquifer Test**

June 2006



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## APPENDICES

- A. Monitoring Well Water Level Data – CD - Under Separate Cover
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## INTRODUCTION

In May and June of 2006, the City of Eagle conducted a 7-day constant rate aquifer pump test of the lower Treasure Valley Aquifer system to determine site specific aquifer conditions under Idaho Department of Water Resource applications for water appropriation 63-32089 and 63-32090. The aquifer test site is located in the western portion of the City of Eagle on the former Quarter Circle D. J. Ranch and Eaglefield Development properties. The test site location is shown in Figure 1.

The aquifer test plan was reviewed and approved by the Idaho Department of Water Resources in December 2005 under drilling permit no. 835987 and drilling permit no. 837870. The aquifer test plan included construction of test well no. 1 located in the proposed Legacy development and test well no. 2 located in the Eaglefield development. Well construction and testing information for the wells are included in Appendix B. Holladay Engineering Company was contracted to conduct the aquifer test and provide engineering services for the project.

The aquifer test was composed of three groundwater monitoring and testing phases. Background monitoring (starting on May 25<sup>th</sup>) was performed for 7-days prior to the pump and test to evaluate water level trends in the aquifer system. A 7-day constant rate pump test was started on June 2<sup>nd</sup> and ending on June 9<sup>th</sup> using a single pumping well and eight monitoring wells (including pumping well) conducted at a pumping rate of 1580 gpm. Seven-days of water level recovery monitoring was performed, immediately following the pump test and ending on June 15<sup>th</sup>. Additional water level recovery data was collected to June 19<sup>th</sup>. A total of eight wells (including the pumping well) were used to monitor groundwater levels during the aquifer test. The monitoring well configuration and well completion information is shown in Figure 1.

The aquifer test generated approximately 300,000 water level measurements that were used to evaluate the aquifer system response to pumping. Transmissivity and storativity values were computed using the Theis Method from drawdown and recovery datasets of monitoring wells completed in the lower aquifer zone.

This report presents data and results from the aquifer test. The following sections describe test procedures, data collection, data corrections, results and analysis.

## AQUIFER TEST PROCEDURES

The aquifer test was designed as a constant rate pump test using 8 wells available at the site (including pumping well) for groundwater level monitoring. The aquifer test project was performed in three phases; background monitoring, constant rate pumping test and recovery monitoring. Each phase was conducted for a minimum duration of 7-days. Background water level monitoring was performed to determine groundwater level trends prior to pump testing. The pump test phase was conducted primarily to determine transmissivity and storativity values in the lower aquifer zone and groundwater responses to pumping at monitoring well locations across the site. The recovery monitoring phase was conducted to determine transmissivity and storativity values in the lower aquifer zone during water level recovery and to characterize the recovery response to pumping at monitoring well locations across the site.

The aquifer test was performed according to the approved aquifer test plan. Two changes were made prior to the start of the test. The pumping well location was moved to test well no. 2 (Eaglefield well) due to the lower capacity of test well no. 1, measured at approximately 1,300 gpm. Well capacity testing and construction information is located in Appendix D. Access to the monitoring well no. 6 (Rick's well) was lost just prior to the start of the test. The City was able to gain limited use of the well during the aquifer test. The data set for monitoring well no. 4 shows limited background and recovery water level data.

### Monitoring Wells

The monitoring wells (including the pumping well) used during the aquifer test are listed below and includes a description of well completion and monitoring configuration. Monitoring well locations are shown in Figure 1. Additional well construction information, well driller's reports and other available information for each monitoring well is located in Appendix B.

1. Monitoring well no. 2 (test well no. 2 - Eaglefield): 12- to 16-inch diameter production well with 12-inch well screens at a depth of 345 ft. to 425 ft. Total well depth is 430 ft. The well is located in the SE  $\frac{1}{4}$  of the NW  $\frac{1}{4}$ , Section 11, T.4N, R.1W and surveyed location is shown on Figure 1. Surveyed ground surface elevation of 2514.15 ft. above mean sea level (amsl). Static water level is artesian. This well was used as the pumping well during the aquifer test. A line-shaft turbine pump (Lyane 12 TLC-4 bowl) was installed for the pump test with an intake depth of 111.22 ft. below ground surface and 100 h.p. 3-phase motor. Two 1-inch diameter pvc plastic sounder tubes were installed from the bottom of the pump column assembly to monitoring ports in motor flange. Water level was monitored with a pressure transducer (Instrumentation N.W. PS-9 with 0-

100 psi sensor) in monitoring port no. 1 and measured by hand using a water level meter and 16 foot sight tube at monitoring port no. 2. A drawing of the wellhead monitoring configuration is located in Appendix B.

2. Monitoring well no. 1 (test well no. 1 - Legacy): 12- to 16-inch diameter well completed with 12-inch diameter screens from a depth of 282 ft. to 352 ft. Total well depth is 362 ft. The well is located in the SW  $\frac{1}{4}$  of the NE  $\frac{1}{4}$ , Section 11, T.4N, R.1W and surveyed well location is shown on Figure 1. Surveyed ground surface elevation of 2512.97 ft. amsl. The distance from the pumping well is 1604.58 ft. Static water level is artesian. The wellhead was configured with two 1 $\frac{1}{4}$ -inch diameter monitoring ports located in the sealed flange plate cover. Water level was monitored with a pressure transducer (Instrumentation NW PT2X with 0-50 psi sensor) in monitoring port no. 1 and measured by hand using a water level meter and 16 foot sight tube at monitoring port no. 2. A drawing of the wellhead monitoring configuration is located in Appendix B.
3. Monitoring well no. 4 (Quarter Circle D. J. Ranch well no. 4): 6-inch diameter irrigation production well completed with an open interval from 235 to 260 feet below ground surface. Total well depth is 260 feet. There is no pump present in the well. The IDWR well driller's report is not available. The well was video inspect using a down-hole camera to determine well completion. Well casing leaks at a constant rate measured at 11.8 gpm at the wellhead surface. Well is located in the SE  $\frac{1}{4}$  of the SW  $\frac{1}{4}$ , Section 3, T.4N, R.1W and surveyed well location is shown on Figure 1. Surveyed ground surface elevation of 2501.86 ft. amsl. The distance from the pumping well is 6276.65 ft. Static water level is artesian. The wellhead was configured with three 1 $\frac{1}{4}$ -inch diameter monitoring ports located in the flange plate cover. Water level was monitored with a pressure transducer (Instrumentation Northwest PT2X with 0-50 psi sensor) in monitoring port no. 1 and measured by hand using a water level meter and 6 foot sight tube at monitoring port no. 2. A 0-15 psi pressure gauge was installed in monitoring port no. 3. A drawing of the wellhead monitoring configuration is located in Appendix B.
4. Monitoring well no. 6 (Quarter Circle D. J. Ranch well no. 6): 16- to 10-inch irrigation production well with line-shaft turbine pump and completed with an open interval from 234 to 395 feet below ground surface. Total well depth is 406 feet. Water level is artesian. Well is located in the SE  $\frac{1}{4}$  of the SW  $\frac{1}{4}$ , Section 2, T.4N, R.1W and surveyed well location is shown on Figure 1. Surveyed ground surface elevation of 2525.84 ft. amsl. The distance from the pumping well is 3339.80 ft. Static water level is artesian. The wellhead was configured with two 1 $\frac{1}{4}$ -inch diameter monitoring ports located in motor support housing. Water level

was monitored with a pressure transducer (Instrumentation N.W. PS-9 with 0-50 psi sensor) in monitoring port no. 1 and measured by hand using a water level meter and 6 foot sight tube at monitoring port no. 2. A drawing of the wellhead monitoring configuration is located in Appendix B.

5. Monitoring well no. 9 (Strata monitoring well no. 1): 1¼ - inch pvc plastic monitoring well completed from 45 to 55 feet below ground surface. Total well depth is 55 feet. Static water level is below ground surface. Well is located in the NW ¼ of the NW ¼, Section 11, T.4N, R.1W and surveyed well location is shown on Figure 1. Surveyed ground surface elevation of 2510.52 ft. amsl. The distance from the pumping well is 3450.28 ft. The measuring point is located on north side of casing. Water level was monitored with a downhole pressure transducer (Instrumentation Northwest PT2X with 0-50 psi sensor) and measured by hand using a water level meter. A drawing of the wellhead monitoring configuration is located in Appendix B.
6. Monitoring well no. 10 (Strata monitoring well no. 1B): 1-inch pvc plastic monitoring well completed from 10 to 15 feet below ground. Total well depth is 15 feet. Static water level is below ground surface. Well is located in the NW ¼ of the NW ¼, Section 11, T.4N, R.1W and surveyed well location is shown on Figure 1. Surveyed ground surface elevation of 2510.52 ft. amsl. The distance from the pumping well is 3425.35 ft. The well measuring point is located on north side of casing. Water level was monitored with a downhole pressure transducer (Instrumentation Northwest PT2X with 0-50 psi sensor) and measured by hand using a water level meter. A drawing of the wellhead monitoring configuration is located in Appendix B.
7. Monitoring well no. 11 (United Water Idaho monitoring well 1A, Hope Lutheran Church): 2-inch pvc plastic monitoring well (part of a nested monitoring well) completed at staggered screen intervals from 280 ft. to 380 ft. below ground surface. Total well depth is 380 feet. Well is located in the SE ¼ of the SE ¼, Section 11, T.4N, R.1W and surveyed well location is shown on Figure 1. Surveyed ground surface elevation of 2518.83 ft. amsl. The distance from the pumping well is 2405.02 ft. Static water level is artesian. The wellhead was configured with an open ½-inch ball valve connected to a tee fitting used as two measuring ports. Port no. 1 was configured a sealed pressure chamber which housed a pressure transducer (Instrumentation Northwest PT2X with 0-50 psi sensor). A 0-15 psi pressure gauge was installed in port no. 2. Groundwater levels were measured by pressure transducer and by pressure gauge. A drawing of the wellhead monitoring configuration is located in Appendix B.

8. Monitoring well no. 12 (United Water Idaho monitoring well 1 B, Hope Lutheran Church): 2-inch pvc plastic monitoring well (part of a nested monitoring well) completed at staggered screen intervals from 400 ft. to 500 ft. below ground surface. Total well depth is 500 feet. Well is located in the SE  $\frac{1}{4}$  of the SE  $\frac{1}{4}$ , Section 11, T.4N, R.1W and surveyed well location is shown on Figure 1. Surveyed ground surface elevation of 2518.83 ft. amsl. The distance from the pumping well is 2405.02 ft. Static water level is artesian. The wellhead was configured with an open  $\frac{1}{2}$ -inch ball valve connected to a tee fitting used as two measuring ports. Port no. 1 was configured a sealed pressure chamber which housed a pressure transducer (Instrumentation Northwest PT2X with 0-50 psi sensor). A 0-30 psi pressure gauge was installed in port no. 2. Groundwater levels were measured by pressure transducer and by pressure gauge. A drawing of the wellhead monitoring configuration is located in Appendix B.

### **Water Level Measurements**

Water level measurements were computed from downhole pressure transducer data collected at each monitoring well location. Transducer pressure measurements were made using Instrumentation Northwest PT2X and PS-9 series transducers with built-in data loggers. PT2X transducers record absolute pressure using a 0-50 psi sensor. Absolute pressure measurements were converted to gauge pressure using direct barometric compensation by subtracting corresponding atmospheric pressure measurements for the same time interval. Barometric pressure was measured and recorded using an Instrumentation Northwest PT2X-BV barometric sensor and data logger unit. Barometric pressure corrections were processed with the Instrumentation Northwest Aqua4plus software. The PS-9 series transducers are designed to measure gauge pressure directly using an atmospheric pressure compensation tube built into the unit and cable assembly.

Monitoring well gauge pressure data was converted to feet of groundwater above the pressure transducer sensor. The data was then converted to depth of groundwater below the measuring point (MP) using the sensor depth setting.

Groundwater level elevation data was calculated using the measured distance of the MP from the surveyed land surface elevation at each monitoring location and depth to groundwater data below the MP. The land surface elevation and location of each monitoring well was surveyed. Survey data for monitoring wells is located in Appendix D. Groundwater measurements and data conversions are shown on the monitoring well data sheet located in Appendix A.

Water level measurements were collected on 1-minute intervals during the background monitoring phase of the test. During the pumping and recovery phases of the test, water level measurements were made on 30-second intervals for the first 2-hour period. After the second hour, measurements were made on 1-minute intervals for the remainder of the test period. Backup hand measurements were made at all monitoring well locations to provide redundancy in the event an electronic transducer failed.

### **Pumping Configuration and Discharge Measurements**

Test well no. 2 (Eaglefield) was used as the pumping well for the aquifer test. A line-shaft turbine pump (Layne 12 TLC-4 bowl) was installed with an intake depth of 111.22 ft. below ground surface. The pump was driven by a 100 h.p. 3-phase G.E. motor powered by a Caterpillar diesel generator. Groundwater was discharged was controlled through an 8-inch diameter gate valve and piped approximately 330 feet to the northwest of the wellhead through 8-inch diameter portable aluminum irrigation pipe. Water flow was measured at the end of the pipeline using a 10-inch diameter circular orifice weir with 4-inch diameter constant discharge orifice plate and manometer tube. Water was discharged into the Middleton Irrigation Association canal system and flowed offsite.

## **AQUIFER TEST RESULTS**

### **Barometric Pressure Monitoring**

Barometric pressure measurement station was set up at the pumping well (test well no. 2) site. Barometric pressure measurements were performed on 1-minute intervals from May 24<sup>th</sup> to June 20<sup>th</sup>. The barometric data was use to correct PT2X transducer measurements. All barometric pressure corrections were processed with the Instrumentation Northwest Aqua4plus software.

Barometric data collected during the test is shown in the barometric data sheets located in Appendix A. A graph of barometric data collected during the test is located in Appendix C.

The pressure transducer data collected at the pumping well (test well no. 2) using an Instrumentation Northwest PS-9 gauge pressure transducer appears to be influenced by atmospheric pressure changes during the test. The water level elevation data graph of test well no. 2 shows a diurnal rise and fall in water level. The PS-9 transducer designed to operate with an atmospheric pressure compensation air tube. The transducer appears to have failed to fully compensate

for barometric pressure changes. The transducer data was not corrected and results presented as recorded.

### **Background Water Level Monitoring**

Background water level monitoring started at 10:00 am on May 25, 2006, except at monitoring well no. 6 where the City did not gain well access until June 2, 2006. Background water level measurements were suspended at test well no. 2 during the installation of the pump on May 25<sup>th</sup> and 26<sup>th</sup>. The pump installation required discharge of artesian flow on May 26, 2006 from 12:30 am to 3:30 for a brief period at 5:15 pm to install the transducer. Artesian flow was discharged into the irrigation canal adjacent to the well site. The pumping system was also tested intermittently between 10:00 am and 12:00 pm on May 31, 2006, which affected water levels in the aquifer during this period.

Water level data collected at each monitoring well is shown on the observation well data sheets located in Appendix A. Background water level data and trends are shown in the monitoring well water level elevation graphs for individual monitoring wells located in Appendix C.

### **Pump Test Water Level Monitoring**

The phase of the aquifer test was started on June 1, 2006 at 10:00 am as scheduled. The pumping rate was adjusted quickly at the discharge gate valve and set to a constant flow rate of 1580 gpm during the test. The discharge flow rate was monitored continuously during start up and on a regular basis (approximately 1-hour intervals) during the later stage of the test. At 3:45 pm on June 1, pumping stopped due to a power failure. A field decision was made to restart the test on June 2 after the aquifer was allowed to recover overnight.

On June 2nd, at 10:00 am, the pump test was restarted for a 7-day period ending on June 9<sup>th</sup> at 10:00 am. At start up the pumping rate was adjusted at the discharge gate valve and set to a constant flow rate of 1580 gpm. The discharge flow rate was monitored continuously during start up and on a regular basis (approximately 1-hour intervals) during the later stage of the test. Two power failures occurred during the test on June 4<sup>th</sup> at 13:11 and June 6<sup>th</sup> at 18:51. In both cases, the pump was restarted immediately, resulting in only a few minutes of non-pumping time. Periodic flow adjustments were made at the discharge gate valve to maintain a constant flow rate of 1580 gpm. The flow rate was observed to drift up to approximately 1% before a flow adjustment was made. In the later portion of the pump test (June 8<sup>th</sup> and 9<sup>th</sup>), water levels in the well were observed to be fluctuating approximately 0.2 inches in the well while performing a hand measurements with an

e-tape. On June 9<sup>th</sup> at 10:00 am, the pump was stopped after seven continuous days of pumping.

Groundwater water level data collected during the test is shown in the observation well data sheets located in Appendix A. Water level elevation graphs for individual monitoring wells are located in Appendix C. Water level drawdown results for test well no. 1 (Legacy), test well no. 2 (Eaglefield), monitoring well 11 (UWI 1A), and monitoring well 12 (UWI 1B) are shown on semi-log plots located in Appendix C.

### **Recovery Water Level Monitoring**

The aquifer recovery period started immediately following the end of pumping on June 9<sup>th</sup> at 10:00 am and continued for seven days to June 16 at 10:00 am. Monitoring well transducers recorded data to June 19<sup>th</sup> until the time each transducer was removed from the well, with the exception of monitoring well no. 6 (Rick's irrigation well) and the pumping well (Test well no. 2). The additional data collected from June 16<sup>th</sup> to June 19<sup>th</sup> was processed and included in the recovery data set.

Several events occurred during the recovery monitoring period. The transducer at monitoring well no. 6 was removed on June 10<sup>th</sup> at 7:39 am. Monitoring well no. 6, an irrigation production owned by Tom Rick, was pumped for irrigation on June 10<sup>th</sup> from approximately 9:15 am to 5:00 pm at an estimated flow rate of 1000 gpm. On June 13<sup>th</sup>, the artesian discharge valve at the pumping well (Test well no. 2) was opened from 10:30 am to 1:08 pm to allow removal of the line-shaft turban pump assembly. The transducer was removed and continued to record measurements while out of the well. The open artesian discharge produced approximately 1000 gpm during this period. Water was discharge into the irrigation canal adjacent to the well site.

Groundwater water level data collected during the test is shown in the observation well data sheets located in Appendix A. Water level elevation graphs for individual monitoring wells are located in Appendix C. Water level recovery results for test well no. 1 (Legacy), test well no. 2 (Eaglefield), monitoring well 11 (UWI 1B), and monitoring well 12 (UWI 1B) are shown on semi-log plots located in Appendix C.

### **AQUIFER TEST DATA ANALYSIS**

Water level data collected from the pumping and recovery periods of the aquifer test from monitoring well no. 1 (test well no. 1), monitoring well 11 (UWI 1A), monitoring well no. 12 (UWI 1B) and the pumping well (test well no. 2) was plotted as drawdown and recovery semi-log and log-log scale graphs. Monitoring well

drawdown and recovery graphs are located in Appendix C. The drawdown and recovery data sets were analyzed using the Theis Method, based on a confined to semi-confined lower aquifer conceptual model for the test site. Transmissivity and storativity values computed from type curve matching of each data set using the Theis Method in AquiferTest 4.0 (Waterloo Hydrogeologic, Inc) computer software and results are listed below in Table 1.

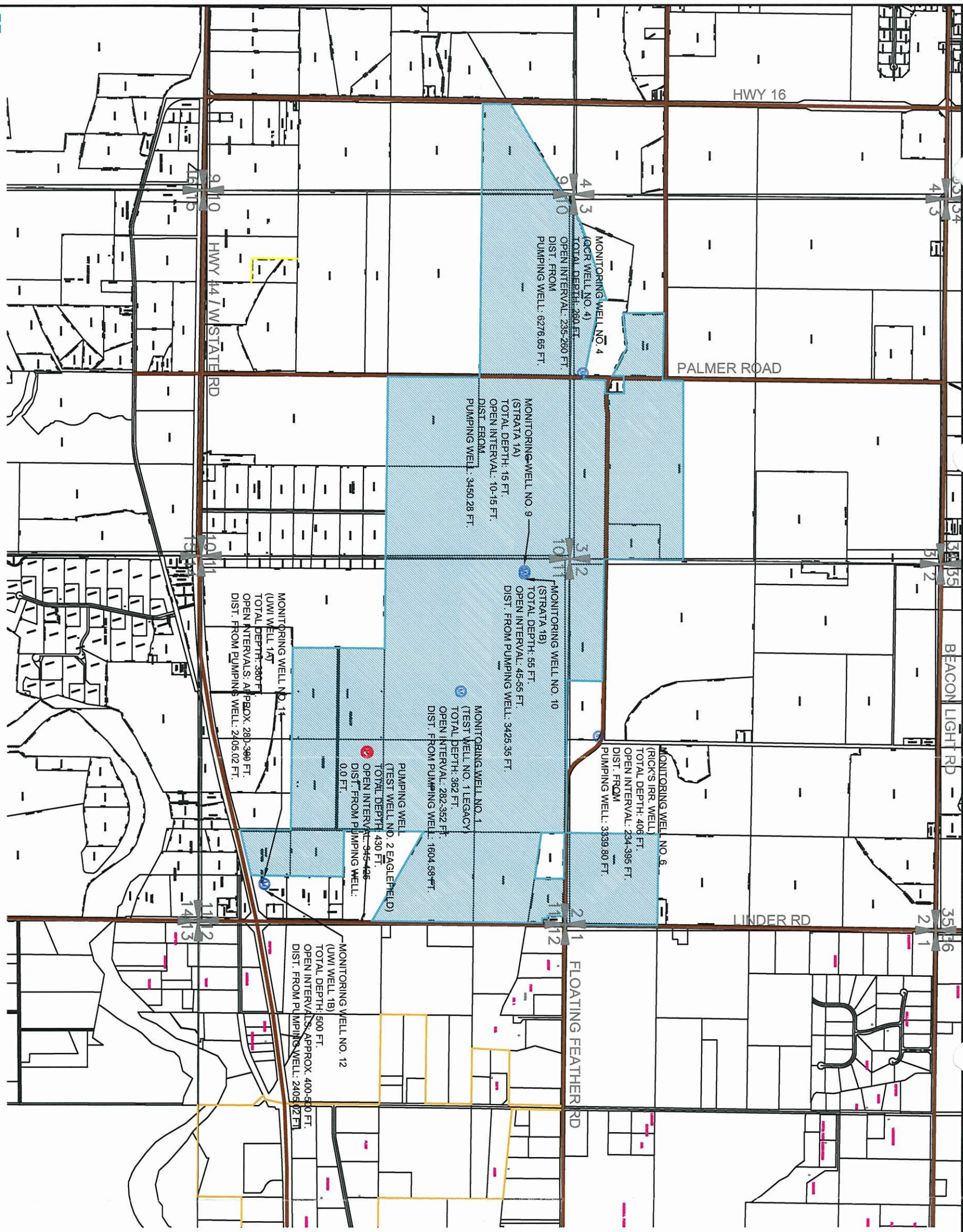
Table 1  
Computed Transmissivity and Storativity Values

Monitoring Well	Data Set	Transmissivity (ft <sup>2</sup> /day)	Storativity
Test well no. 1 (Legacy)	Drawdown	2.01 x 10 <sup>4</sup> ✓	1.30 x 10 <sup>-2</sup>
Test well No. 2 (pumping well)	Drawdown	2.00 x 10 <sup>4</sup>	---
Monitoring well no. 11 (UWI 1A)	Drawdown	1.85 x 10 <sup>4</sup>	6.62 x 10 <sup>-4</sup>
Monitoring well no. 12 (UWI 1B)	Drawdown	1.80 x 10 <sup>4</sup>	8.00 x 10 <sup>-4</sup>
Test well no. 1 (Legacy)	Recovery	1.95 x 10 <sup>4</sup> ✓	1.58 x 10 <sup>-2</sup>
Test well No. 2 (pumping well)	Recovery	1.75 x 10 <sup>4</sup>	---
Monitoring well no. 11 (UWI 1A)	Recovery	1.77 x 10 <sup>4</sup>	7.80 x 10 <sup>-4</sup>
Monitoring well no. 12 (UWI 1B)	Recovery	1.80 x 10 <sup>4</sup>	7.90 x 10 <sup>-4</sup>
Average Value	Test	1.87 x 10 <sup>4</sup>	5.31 x 10 <sup>-3</sup>

Water level data collected from the pumping and recovery periods of the aquifer test from monitoring well no. 1 (test well no. 1), monitoring well 11 (UWI 1A), monitoring well no. 12 (UWI 1B) and the pumping well (test well no. 2) was plotted as drawdown and recovery semi-log and log-log scale graphs. Monitoring well drawdown and recovery graphs are located in Appendix C. The drawdown and recovery data sets were analyzed using the Theis Method, based on a confined to semi-confined lower aquifer conceptual model for the test site. Transmissivity and storativity values computed from type curve matching of each data set using the Theis Method in AquiferTest 4.0 (Waterloo Hydrogeologic, Inc) computer software and results are listed below in Table 1.

In the Theis analysis type curve fit, data points influenced by well interference or discharge of artesian flow were given a lower weighted value or excluded from the data set to obtain a more representative type curve fit. The Theis analysis graphs showing calculated transmissivity and storativity values for each data set are located in Appendix C. The computed average value for transmissivity is 1.87 x 10<sup>4</sup> ft<sup>2</sup>/day (139,886 gal/day/ft). The average value for storativity is 5.31 x 10<sup>-3</sup>.

CITY OF EAGLE  
AQUIFER TEST WELL MAP  
FIGURE - 1



- LEGEND:
- PROPERTY OWNERSHIP
  - PROPOSED DEVELOPMENT
  - AQUIFER TEST WELLS
  - MONITORING WELL
  - PUMPING WELL

- ROADS
- PARCEL LINES
- SECTION LINES
- SECTION CORNERS



NOTE:  
1) BASEMAP IS ADA AND CANYON COUNTY (2005)

This map represents a compilation of public information from diverse records gathered by the City of Eagle and Holladay Engineering Company. The purpose for which this map is prepared is an overall general representation of positional relationships, and not a definitive description of location, distance, or area. The City of Eagle and Holladay Engineering Company CANNOT AND DO NOT GUARANTEE the absence of errors or the correctness of all information furnished to them for the preparation of this map.

# **APPENDIX A**

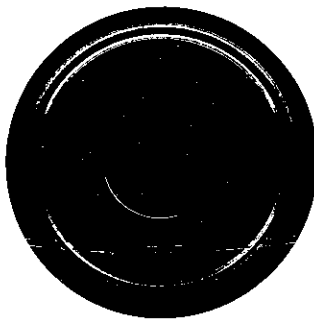
(Computer Disk Enclosed)

## **MONITORING WELL WATER LEVEL DATA**

1. Test Well 2 (Eaglefield)(Pumping Well)
2. Test Well 1 (Legacy)
3. Monitoring Well 4 (QCR 4)
4. Monitoring Well 6 (Rick's)
5. Monitoring Well 9 (Strata 1A)
6. Monitoring Well 10 (Strata 1B)
7. Monitoring Well 11 (UWI 1A)
8. Monitoring Well 12 (UWI 1B)
9. Barometer

# **Appendix A**

**Monitoring Well Water Level Data  
EG061204**



**Holladay Engineering, Co.**

**Universal**  
Data Corp.

39661

## **APPENDIX B**

### **MONITORING WELL INFORMATION**

1. Test Well 2 (Eaglefield)(Pumping Well)
2. Test Well 1 (Legacy)
3. Monitoring Well 4 (QCR 4)
4. Monitoring Well 6 (Rick's)
5. Monitoring Well 9 (Strata 1A)
6. Monitoring Well 10 (Strata 1B)
7. Monitoring Well 11 (UWI 1A)
8. Monitoring Well 12 (UWI 1B)

## Appendix B.1

TEST WELL 2 (Eaglefield)(Pumping Well)

## WELL DRILLER'S REPORT

Office Use Only			
Inspected by _____			
Twp _____	Rge _____	Sec _____	
1/4		1/4	1/4
Lat: _____		Long: _____	

1. WELL TAG NO. D 0042405  
DRILLING PERMIT NO. 892605 - 837870  
Water Right or Injection Well No. 63-32089, 63-32090

## 2. OWNER:

Name CITY OF EAGLE  
Address 310 E. STATE ST.  
City EAGLE State CO Zip 83616

## 3. LOCATION OF WELL by legal description:

You must provide address or Lot, Blk, Sub. or Directions to well. WELL #2

Twp. 4 North ☒ or South ☐ (EAGLEFIELD)  
Rge. 1 East ☐ or West ☒  
Sec. 11 1/4 NW 1/4 SE 1/4  
Gov't Lot \_\_\_\_\_ County ADAMS

Lat: \_\_\_\_\_ Long: \_\_\_\_\_  
Address of Well Site 50' W of INTERSECTION of WATLOCK DR +  
GOLDEN CANYON WALK City EAGLE  
(Show at least 100 feet to road - Distance to Road or Landmark)  
US 884 Blk. R Sub. Name EAGLEFIELD ESTATES

## 4. USE:

☐ Domestic ☒ Municipal ☐ Monitor ☐ Irrigation  
☐ Thermal ☐ Injection ☒ Other TEST

## 5. TYPE OF WORK check all that apply

(Replacement etc.)

☒ New Well ☐ Modify ☐ Abandonment ☐ Other \_\_\_\_\_

## 6. DRILL METHOD:

☐ Air Rotary ☐ Cable ☐ Mud Rotary ☒ Other REVERSE

## 7. SEALING PROCEDURES

Seal Material	From	To	Weight / Volume	Seal Placement Method
CEMENT GROUT	0	320	25/480	PUMPED FROM BOTTOM TO TOP

Was drive shoe used? ☐ Y ☒ N Shoe Depth(s) \_\_\_\_\_

Was drive shoe seal tested? ☐ Y ☐ N How? \_\_\_\_\_

8. CASING/LINER: 280' 16" X 12" REDUCER.

Diameter	From	To	Gauge	Material	Casing	Liner	Welded	Threaded
16	45	280	375	STEEL	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
12	281	345	375	STEEL	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

Length of Headpipe \_\_\_\_\_ Length of Tailpipe 5'

Packer ☐ Y ☒ N Type \_\_\_\_\_

## 9. PERFORATIONS/SCREENS PACKER TYPE

Perforation Method \_\_\_\_\_

Screen Type & Method of Installation JOHNSON WIRE WRAP

From	To	Slot Size	Number	Diameter	Material	Casing	Liner
245	425	.030		12	S.S.	<input checked="" type="checkbox"/>	<input type="checkbox"/>

## 10. FILTER PACK

Filter Material	From	To	Weight / Volume	Placement Method
#16-20 COARSE SAND	320	425	30,000	DRY POUR

## 11. STATIC WATER LEVEL OR ARTESIAN PRESSURE:

ft. below ground \_\_\_\_\_ Artesian pressure 8.1 lb.

Depth flow encountered \_\_\_\_\_ ft. Describe access port or control devices:

FLANGE CAP W/ 2-1/4" PIPE PLUGS

## 12. WELL TESTS:

☐ Pump ☐ Baller ☐ Air ☐ Flowing Artesian

Yield gnl./min.	Drawdown	Pumping Level	Time
1123 gpm		45.2'	10 MIN

Water Temp. \_\_\_\_\_ Bottom hole temp. \_\_\_\_\_

Water Quality test or comments: \_\_\_\_\_

## 13. LITHOLOGIC LOG: (Describe repairs or abandonment) Water

Bore Dia.	From	To	Remarks: Lithology, Water Quality & Temperature	Y	N
28	0	3	TOP SOIL		
	3	28	RIVER GRAVEL		
	28	48	TAN CLAY		
	48	56	SAND		
	56	63	RUSTY TAN CLAY		
	63	135	SAND W/SM TAN CLAY STRAKS		
	135	183	BN CLAY W/SAND STRAKS		
	183	189	FINE-MED SAND		
	189	194	TAN CLAY		
	194	215	SAND SH BEVEL		
	215	232	CLAY		
	232	290	SAND FINE-MED W/CLAY STRAKS		
	290	297	SAND-FINE		
	297	339	BR. DUSTY GREEN-MED CLAY		
	339	436	COARSE SAND, SOME TAN CLAY		
	436	444	BN RUSTY CLAY		

Completed Depth 430' (Measurable)

Date: Started 3-21-06 Completed 5-9-06

## 14. DRILLER'S CERTIFICATION

I/We certify that all minimum well construction standards were complied with at the time the rig was removed.

Company Name RIVERSIDE INC Firm No. 333

Principal Driller [Signature] Date 5-22-06

and Driller or Operator II Tustin Chavez Date 5-22-06

Operator I \_\_\_\_\_ Date \_\_\_\_\_

Principal Driller and Rig Operator Required.  
Operator I must have signature of Driller/Operator II.

FORWARD WHITE COPY TO WATER RESOURCES



April 8, 2006

Peter Harris  
Eaglefield, LLC  
6951 Duncan Lane  
Boise, ID 83714

Subject: Final Design for Eaglefield Well No. 1 (City of Eagle Test Well 4)

Dear Peter:

Drilling of the municipal test well borehole at Eaglefield was completed to approximately 444 feet on April 7, 2006. The borehole is nominal 20-inch diameter and was advanced using the reverse rotary method. The drill cuttings consist primarily of brown sand, tan clay, and brown clay above 438 feet. Blue-gray clay was encountered from 438 to 444 feet.

Geophysical logging was conducted on April 7. Strata Data, Inc. from Casper, Wyoming, was the geophysical contractor. Logs run include natural gamma radiation, spontaneous potential, single point resistivity, 16-inch normal resistivity, and 64-inch normal resistivity.

Drill cuttings and geophysical logs document the presence of a potentially productive sand layer from 330 feet to 433 feet. This sand layer corresponds with the target interval for this well, and we propose completing the well with screens extending from 345 to 425 feet.

The completed well will include 16-inch casing from approximately 5 feet above ground surface to 280 feet, 12-inch casing from 280 feet to 345 feet, 12-inch well screen from 345 feet to 425 feet, and 12-inch tail pipe with plate bottom from 425 feet to 430 feet. A 16-inch by 12-inch welded reducer will be used to connect the 12-inch and 16-inch casing strings. The invert of the 10-inch artesian by-pass line will be approximately 2.5 feet above existing ground surface.

No. 6-9 Colorado Silica Sand filter pack will be installed in the borehole annulus from 320 to 430 feet. Estimated volume is approximately 5 cubic yards.

Bentonite chips will be used as surface seal material. The chips will extend from 320 feet to ground surface. Estimated volume is approximately 34 cubic yards. Prior to installation of casing and screen, the borehole from ground surface to 300 feet will be reamed to 28-inch diameter to facilitate installation of the bentonite surface seal.

We are still discussing well development options with Riverside. Our current thinking is to develop the well initially by bailing, air lifting, or pumping until artesian flow is

achieved. After the initial artesian flow clears, development will continue by shutting in the well, pressurizing the casing with compressed air, and then releasing pressure to allow artesian flow. This procedure can be repeated until the artesian flow clears. Air pressures should start low, and gradually increase up to 100 psi. The artesian flow rate and shut-in pressure following initial development will allow a preliminary determination of well capacity as soon as possible. Final development will be completed using the test pump.

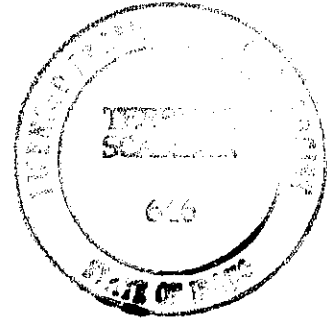
A well design schematic, drill log, and field geophysical logs are enclosed for your records.

Please contact me with any questions.

Sincerely,



Terry M. Scanlan, P.E., P.G.

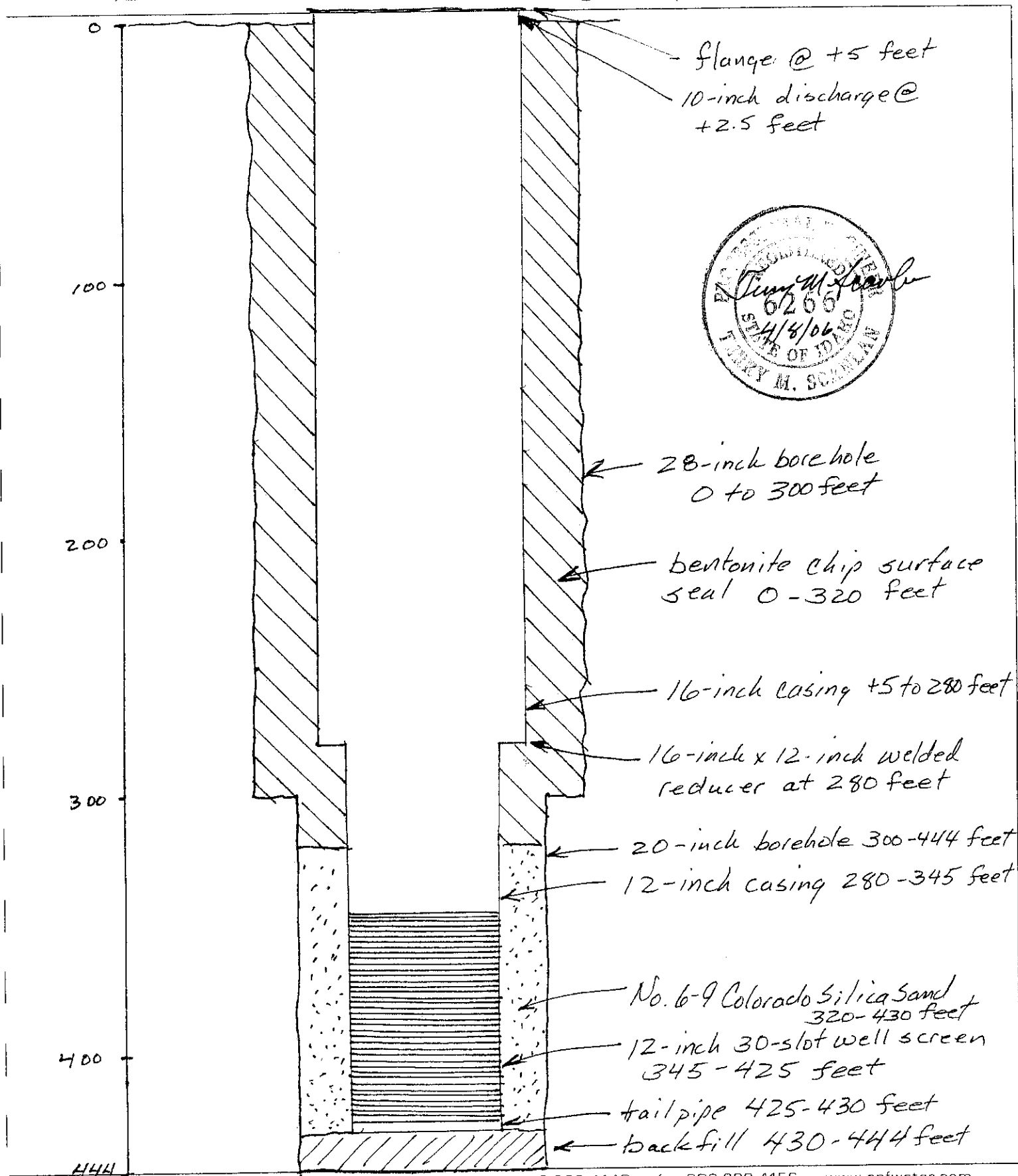


cc: Terry Daugherty – Riverside, Inc.  
Rob Whitney – Idaho Department of Water Resources  
Chris Duncan – Holladay Engineering

Attachments: Well Schematic  
Drill Log  
Geophysical Logs



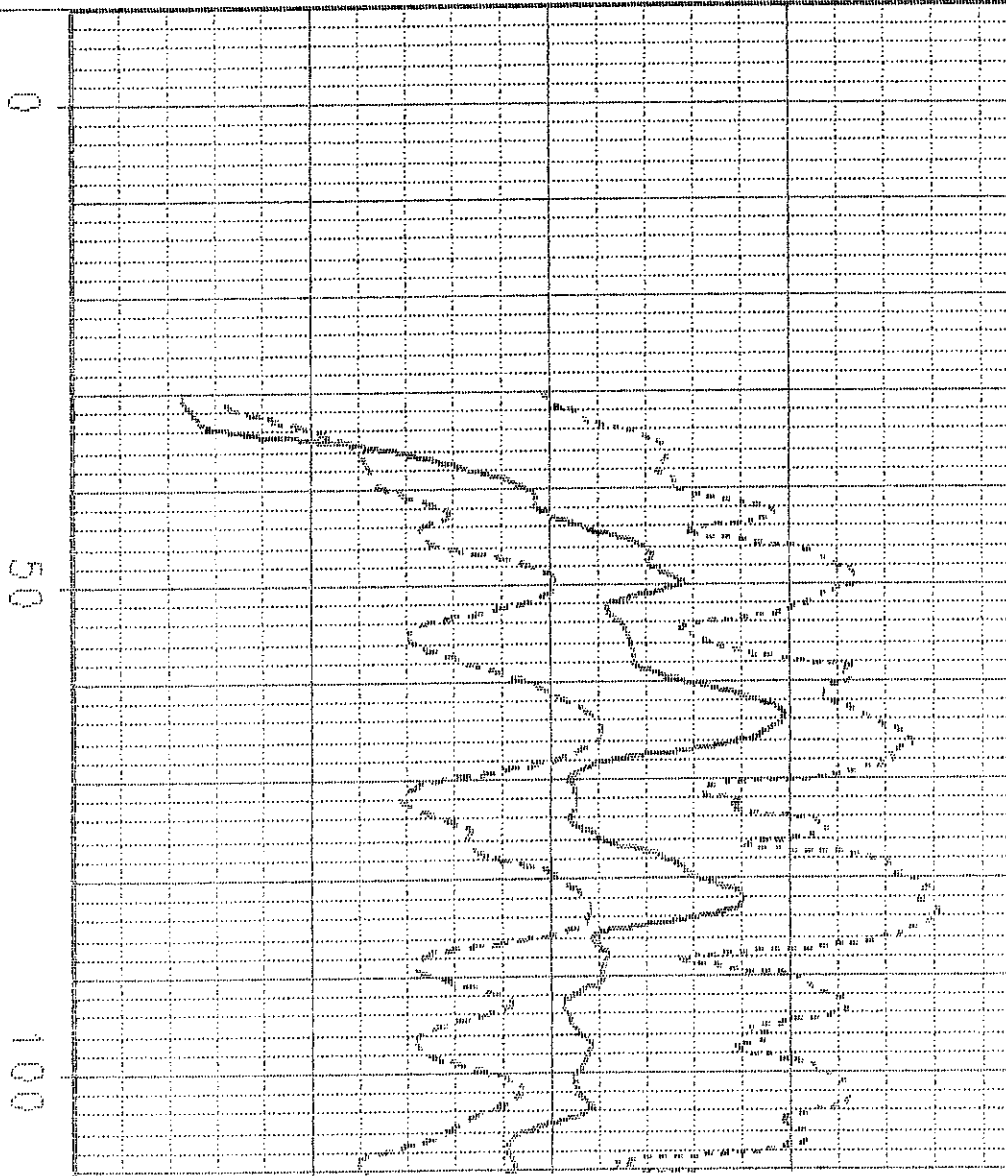
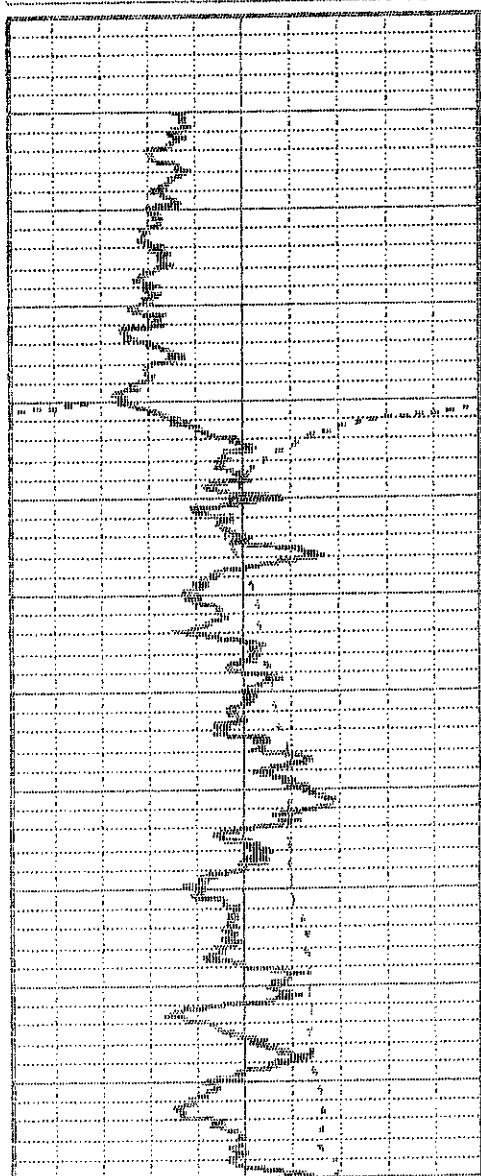
By TS Date 4/8/06 Client Eaglefield Sheet 1 of 1  
Chkd By \_\_\_\_\_ Description Final Well Design - Eagle Well #4 Job No 421.00.10

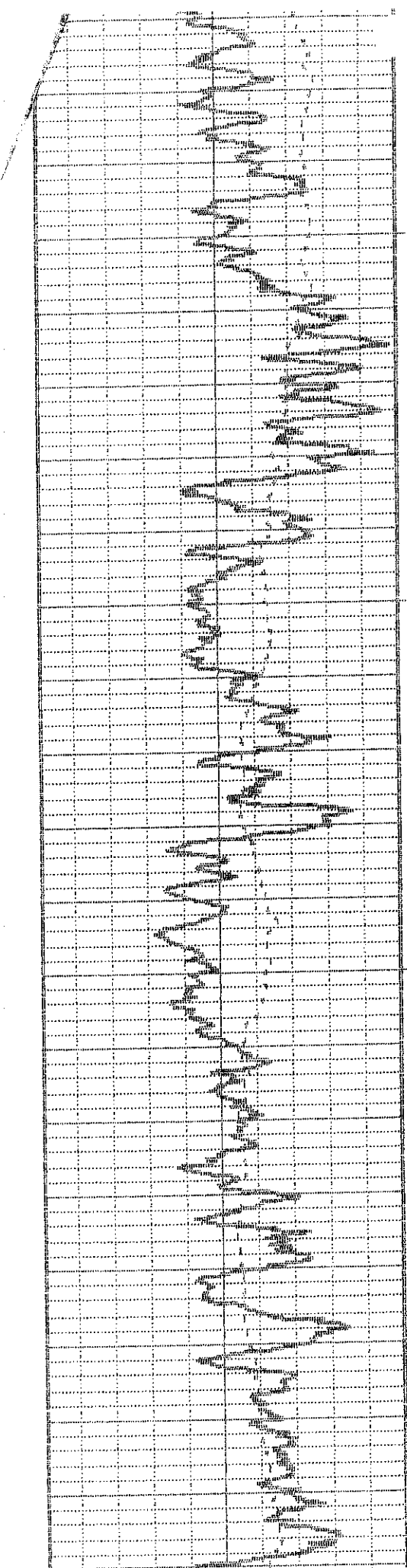


# EAGLE SPORTS FIELD TEST WELL 2

SPONTANEOUS POTENTIAL  
 -220  
 NATURAL GAMMA  
 130

54" NORMAL RESISTIVITY  
 OHM-M 250  
 15" NORMAL RESISTIVITY  
 OHM-M 250  
 POINT RESISTIVITY  
 OHMS 100



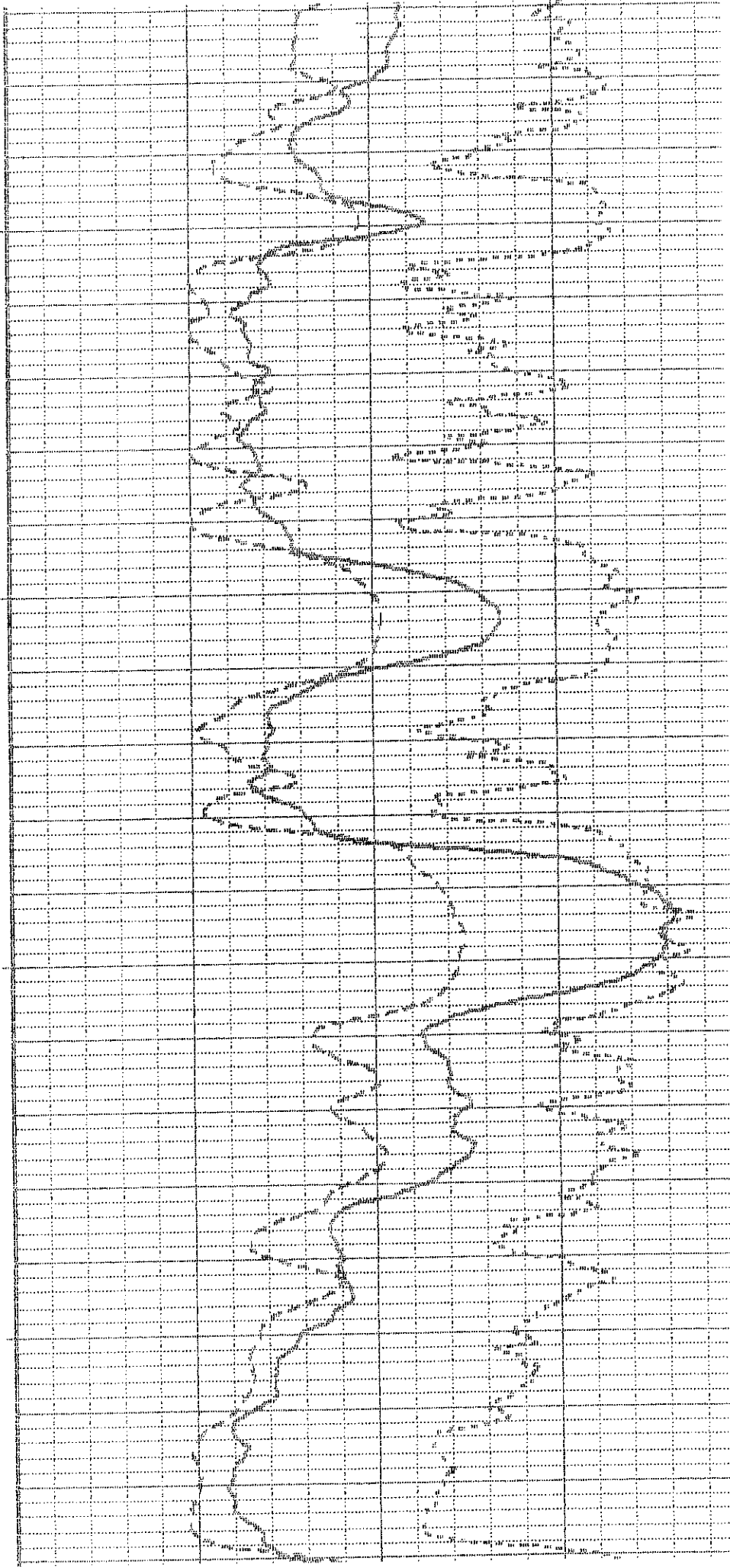


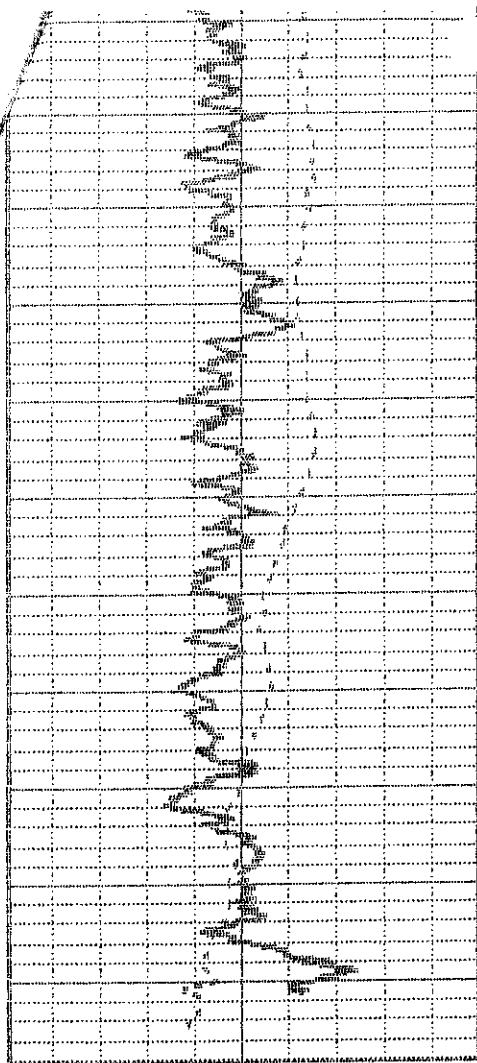
150

200

250

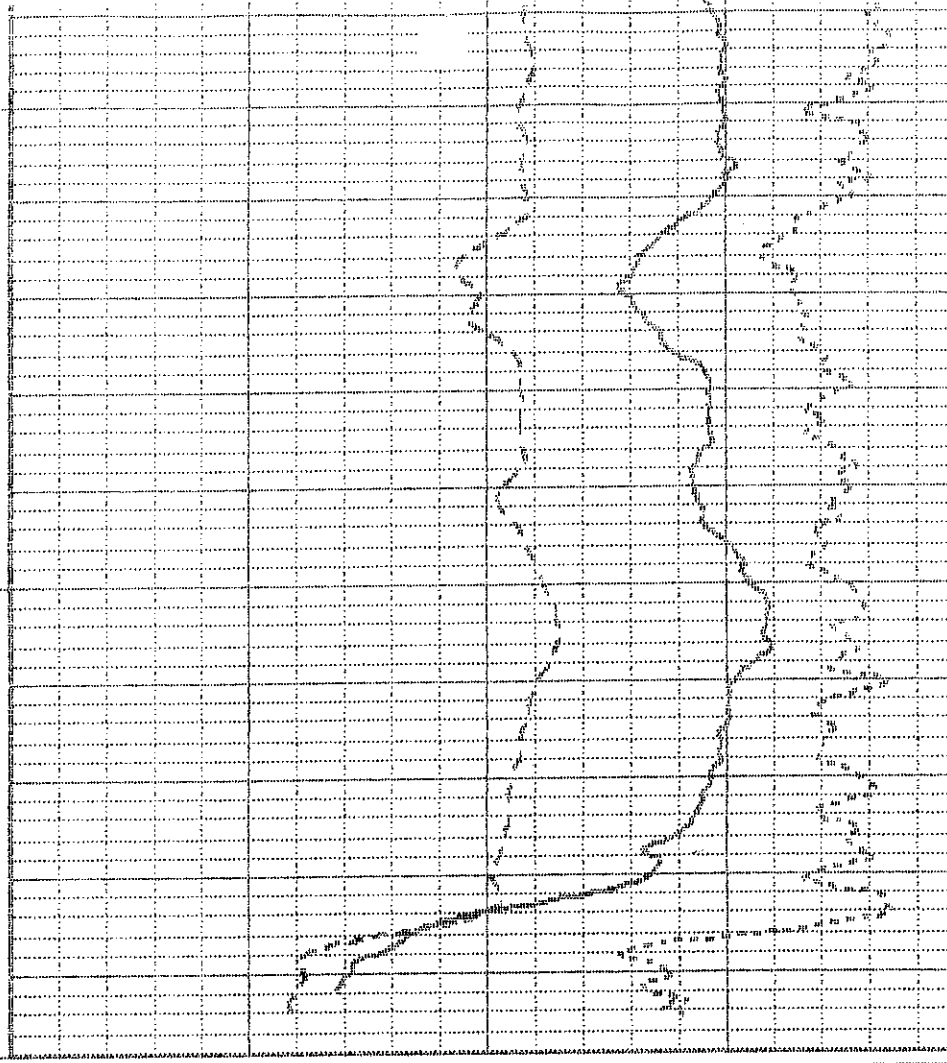
300





350

400



NATURAL GAMMA  
API UNITS 150  
SPONTANEOUS POTENTIAL  
mV -250 -150

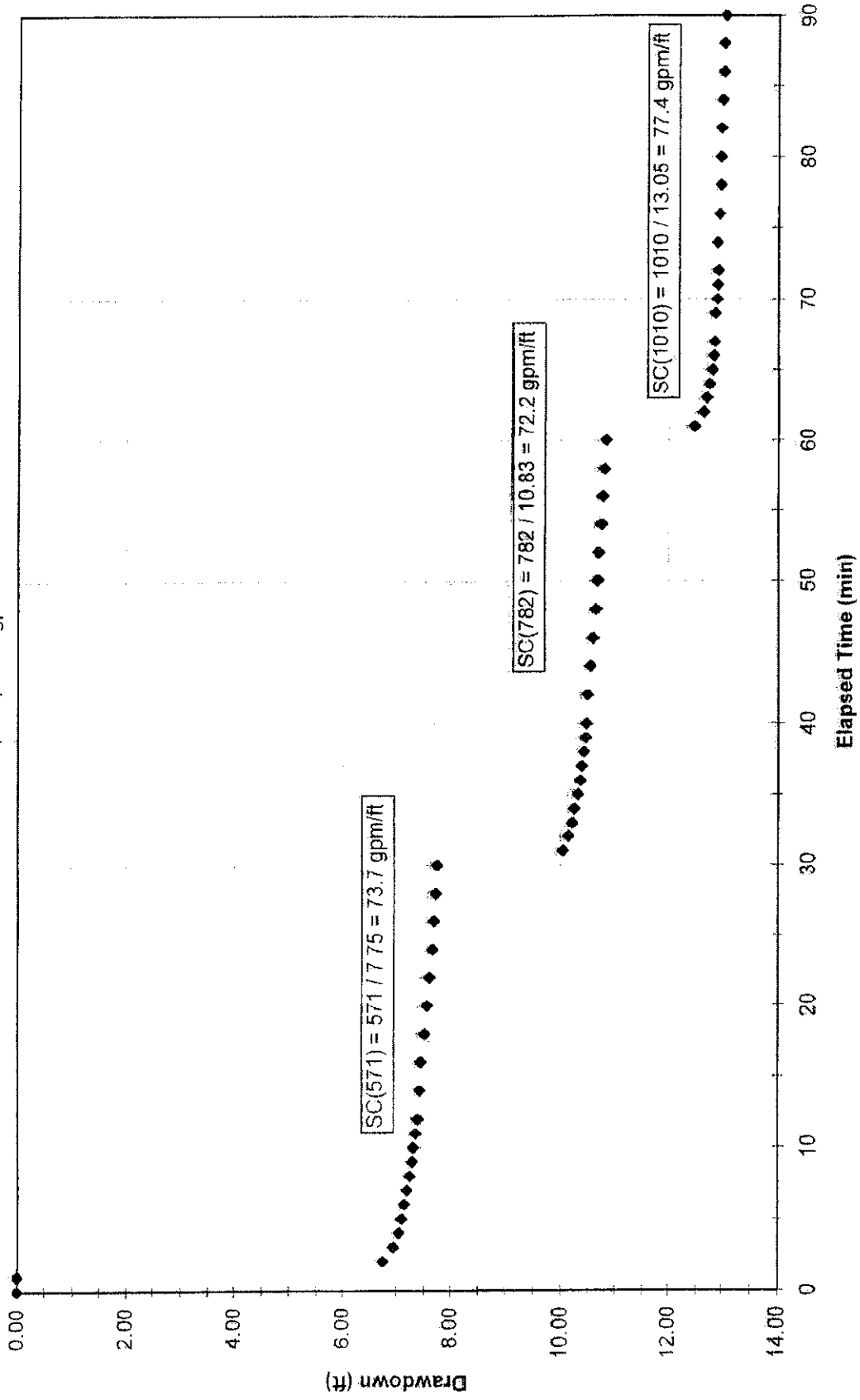
POINT RESISTIVITY  
OHMS 100  
16" NORMAL RESISTIVITY  
OHM-M 250  
64" NORMAL RESISTIVITY  
OHM-M 250

EAGLE SPORTS FIELD TEST WELL 2

# Eaglefield Well No. 1 Step Test

Test Date: 5/16/06

Q = 571, 782, 1010 gpm



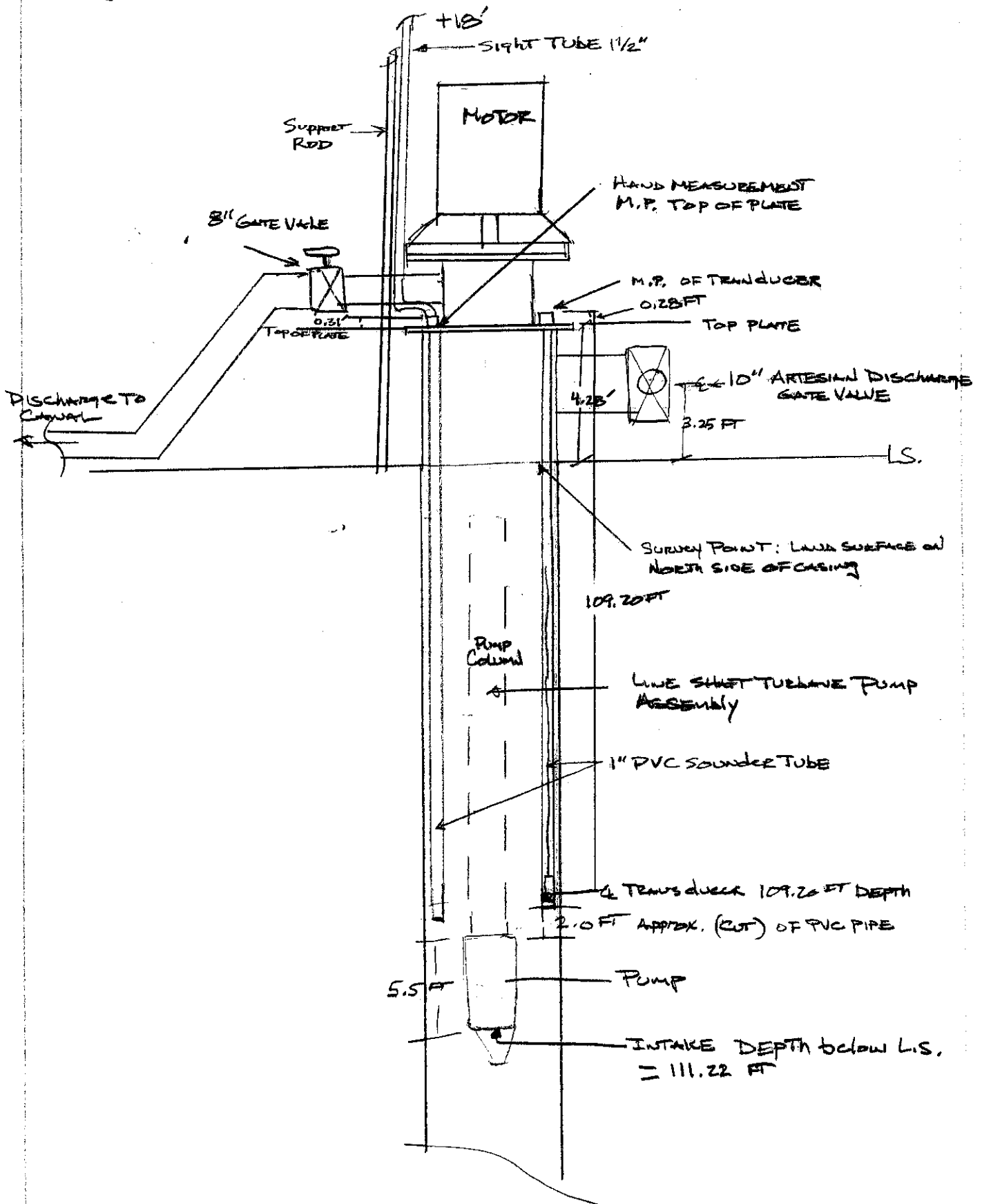
Eaglefield Well No. 1 Step Test, Q = 571, 782, 1010 gpm					
Test conducted by: SPF Water					
Flow measured by: 10x7 orifice, h = 8, 15, 25-inches					
Water level measured by: manometer, all water levels above ground surface					
Date	Time	WL (ft)	Elapsed Time (min)	DD (ft)	Remarks
5/16	12:56	18.75			static
5/16	13:00		0	0.00	open gate valve
5/16	13:01	18.75	1	0.00	adjust h=8"
5/16	13:02	12.00	2	6.75	
5/16	13:03	11.80	3	6.95	
5/16	13:04	11.70	4	7.05	
5/16	13:05	11.65	5	7.10	
5/16	13:06	11.60	6	7.15	
5/16	13:07	11.55	7	7.20	
5/16	13:08	11.50	8	7.25	
5/16	13:09	11.45	9	7.30	
5/16	13:10	11.43	10	7.32	
5/16	13:11	11.39	11	7.36	
5/16	13:12	11.35	12	7.40	T=16.3, pH=7.44, EC/SC=208.1/244.0
5/16	13:14	11.32	14	7.43	
5/16	13:16	11.30	16	7.45	
5/16	13:18	11.23	18	7.52	T=15.9, pH=7.46, EC/SC=209.9/246.4
5/16	13:20	11.19	20	7.56	
5/16	13:22	11.14	22	7.61	
5/16	13:24	11.09	24	7.66	
5/16	13:26	11.06	26	7.69	
5/16	13:28	11.03	28	7.72	T=16.1, pH=7.45, EC/SC=209.9/246.4
5/16	13:30	11.00	30	7.75	increase h=15"
5/16	13:31	8.70	31	10.05	
5/16	13:32	8.60	32	10.15	
5/16	13:33	8.53	33	10.22	
5/16	13:34	8.49	34	10.26	
5/16	13:35	8.42	35	10.33	
5/16	13:36	8.38	36	10.37	
5/16	13:37	8.35	37	10.40	
5/16	13:38	8.31	38	10.44	
5/16	13:39	8.28	39	10.47	difficult to read manometer, behind duct tape
5/16	13:40	8.27	40	10.48	
5/16	13:42	8.25	42	10.50	
5/16	13:44	8.20	44	10.55	T=16.1, EC/SC=208.8/245.6
5/16	13:46	8.15	46	10.60	
5/16	13:48	8.10	48	10.65	
5/16	13:50	8.07	50	10.68	
5/16	13:52	8.05	52	10.70	
5/16	13:54	8.00	54	10.75	T=16.0, pH=7.38, EC/SC=208.2/245.4
5/16	13:56	7.98	56	10.77	
5/16	13:58	7.95	58	10.80	T=16.0, pH=7.37, EC/SC=208.0/244.9
5/16	14:00	7.92	60	10.83	
5/16	14:01	6.28	61	12.47	increase h=25"
5/16	14:02	6.11	62	12.64	

Date	Time	WL (ft)	Elapsed Time (min)	DD (ft)	Remarks
5/16	14:03	6.05	63	12.70	
5/16	14:04	6.00	64	12.75	
5/16	14:05	5.94	65	12.81	
5/16	14:06	5.92	66	12.83	
5/16	14:07	5.90	67	12.85	check on flow along discharge channel, misread reading
5/16	14:09	5.89	69	12.86	
5/16	14:10	5.85	70	12.90	adjust gate valve couple of turns
5/16	14:11	5.84	71	12.91	
5/16	14:12	5.83	72	12.92	
5/16	14:14	5.85	74	12.90	T=16.0, pH=7.43, EC/SC=208.6/245.6
5/16	14:16	5.81	76	12.94	
5/16	14:18	5.79	78	12.96	
5/16	14:20	5.79	80	12.96	T=16.0, pH=7.42, EC/SC=208.7/245.7
5/16	14:22	5.78	82	12.97	
5/16	14:24	5.75	84	13.00	
5/16	14:26	5.73	86	13.02	
5/16	14:28	5.73	88	13.02	
5/16	14:30	5.70	90	13.05	closed gate valve

# TEST Well No. 2 (EAGLEFIELD)

Pumping Well

EAGLE 7-DAY AQUIFER TEST



## Appendix B.2

### TEST WELL 1 (Legacy)

IDAHO DEPARTMENT OF WATER RESOURCES  
WELL DRILLER'S REPORT

Office Use Only			
Well ID No.			
Inspected by			
Twp	Rge	Sec	
1/4	1/4	1/4	
Lat:		Long:	

1. WELL TAG NO. D 0041980  
DRILLING PERMIT NO. 890994-835987  
Water Right or Injection Well No. 63-32089, 63-32090

## 2. OWNER:

Name CITY OF EAGLE  
Address P.O. Box 1520  
City EAGLE State ID Zip 83616

3. LOCATION OF WELL by legal description: LEGACY WELL #2

You must provide address or Lot, Blk. Sub. or Directions to well.

Twp. 4 North ☒ or South ☐  
Rge. 1 East ☐ or West ☒  
Sec. 11 1/4 SE 1/4 NW 1/4  
Gov't Lot \_\_\_\_\_  
County ADA State ID  
Let: \_\_\_\_\_ Long: \_\_\_\_\_  
Address of Well Site QUARTER CIRCLE DR RANCH, WEST  
OF EAGLE ROAD City EAGLE  
(Give in town names or road & direction to well or landmark)  
Lt. \_\_\_\_\_ Blk. \_\_\_\_\_ Sub. Name \_\_\_\_\_

## 4. USE:

☐ Domestic ☐ Municipal ☐ Monitor ☐ Irrigation  
☐ Thermal ☐ Injection ☒ Other TEST

## 5. TYPE OF WORK check all that apply (Replacement etc.)

☒ New Well ☐ Modify ☐ Abandonment ☐ Other \_\_\_\_\_

## 6. DRILL METHOD:

☐ Air Rotary ☐ Cable ☐ Mud Rotary ☒ Other REVERSE

## 7. SEALING PROCEDURES

Seal Material	From	To	Weight / Volume	Seal Placement Method
CEMENT GROUT	0	272	24 gals	PUMPED BOTTOM TO TOP
BENTONITE	360	415	12,000	DRY POUR

Was drive shoe used? ☐ Y ☒ N Shoe Depth(s): \_\_\_\_\_  
Was drive shoe seal tested? ☐ Y ☒ N How? \_\_\_\_\_

8. CASING/LINER: 16" X 12" REDUCER @ 180' TO 181'

Diameter	From	To	Gauge	Material	Casing	Liner	Welded	Threaded
16	4	180	375	STEEL	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
12	181	282	375	STEEL	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>

Length of Headpipe \_\_\_\_\_ Length of Tailpipe 5'  
Packer ☐ Y ☒ N Type \_\_\_\_\_

## 9. PERFORATIONS/SCREENS PACKER TYPE

Perforation Method	Screen Type & Method of Installation
	<u>JOHNSON WIRE WRAP</u>

From	To	Slot Size	Number	Diameter	Material	Casing	Liner
282	352	.030		12	S.S.	<input checked="" type="checkbox"/>	<input type="checkbox"/>

## 10. FILTER PACK

Filter Material	From	To	Weight / Volume	Placement Method
#8-12 SAND	272	360	24,000	DRY POUR

## 11. STATIC WATER LEVEL OR ARTESIAN PRESSURE:

\_\_\_\_\_ ft. below ground Artesian pressure 6 lb.  
Depth flow encountered \_\_\_\_\_ ft. Describe access port or control devices:  
FLANGED CAP w/ 1 1/4" PIPE PLUG

## 12. WELL TESTS:

Yield gal/min.	Drawdown	Pumping Level	Time
1300 gpm	149'	135'	4 YEARS

Water Temp. \_\_\_\_\_ Bottom hole temp. \_\_\_\_\_

Water Quality test or comments: \_\_\_\_\_

Depth first Water Encounter \_\_\_\_\_

## 13. LITHOLOGIC LOG: (Describe repairs or abandonment)

Bottom Dis.	From	To	Remarks: Lithology, Water Quality & Temperature	Y	N
24	0	4	TOP SOIL		
	4	7	CLAY		
	7	11	SAND		
	11	28	SAND, RIVER GRAVELS		
	28	60	BEN CLAY		
	60	63	SAND		
	63	72	BEN CLAY		
	72	92	SAND		
	92	94	BEN CLAY		
	94	166	SAND w/ BEN CLAY STREAKS		
	166	174	CLAY		
	174	178	SAND		
	178	181	CLAY		
	181	183	SAND, CLAY STREAKS		
	183	257	SAND		
	257	264	CLAY		
	264	274	SAND w/ CLAY STREAKS		
	274	353	SAND		
	353	360	BLUE CLAY		
	360	384	BLUE CLAY		
	384	387	SAND		
	387	419	BLUE-GRAY CLAY		
	419	444	SAND		
	444	459	BLUE-GRAY CLAY		
	459	493	SAND w/ CLAY LAYERS		
	493	501	BLUE-GRAY CLAY		
	501	505	CEMENTED SAND		
	505	513	BLUE-GRAY CLAY		
			NOTE: BORE HOLE ABANDONMENT FROM 415' TO 513' WITH DRILL CUTTINGS		

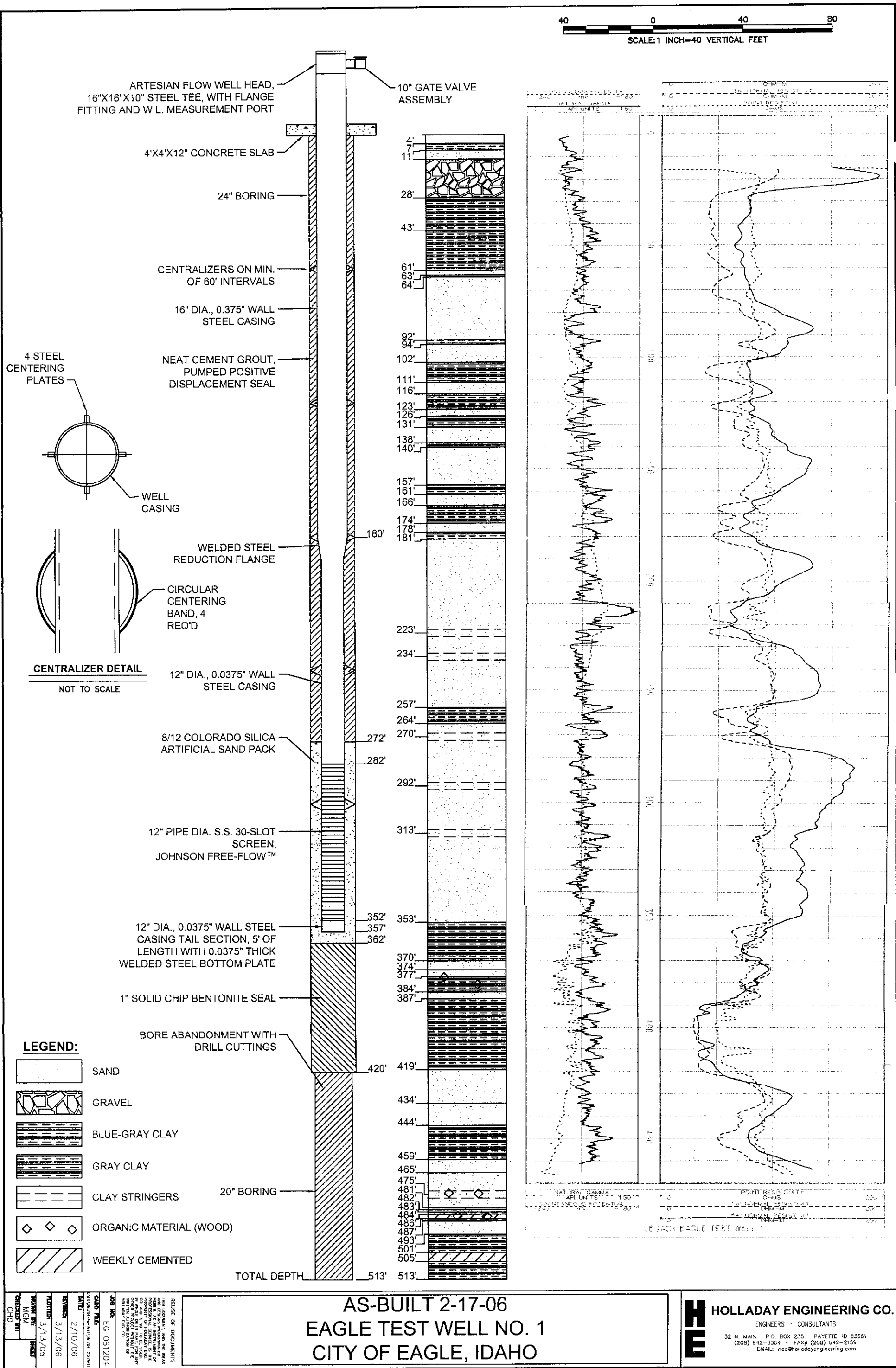
Completed Depth 357' (Measurable)  
Date: Started 1-24-06 Completed 4-17-06

## 14. DRILLER'S CERTIFICATION

I/We certify that all minimum well construction standards were complied with at the time the rig was removed.

Company Name RIVERSIDE INC Firm No. 333  
Principal Driller [Signature] Date 5-15-06  
and  
Driller or Operator II [Signature] Date \_\_\_\_\_  
Operator I [Signature] Date 5-15-06  
Principal Driller and Rig Operator Required.  
Operator I must have signature of Driller/Operator II.

FORWARD WHITE COPY TO WATER RESOURCES



# GEO TECHNICAL BOREHOLE LOG

HOLLADAY ENGINEERING COMPANY

BOREHOLE ID: TEST Well No. 1 PROJECT NO.: EG 061204 PAGE: 1 of 6  
 (LEGACY)  
 PROJECT NAME: CITY OF EAGLE LOCATION: QUARTER CIRCLE DT Ranch SITE LOCATION: SE, NW, S. 11, T. 4N, R. 1W  
WR 63-32000 AND  
WR 63-32000 EAGLE, ID  
 COLLAR EL: \_\_\_\_\_ NORTHING: \_\_\_\_\_ EASTING: \_\_\_\_\_  
 HOLE DIAMETER: 18", 20" DRILL METHOD: REVERSE ROTARY DRILL MODEL: JED-A  
 (BIT) MUD  
 DRILLER: RIVERSIDE, INC. LOGGER: C.D. RIVERSIDE, INC. STATIC WATER DEPTH: \_\_\_\_\_  
(JEFF HASH)

Depth (ft)	Date Time	Induration & Color	Soil or Rock Description	Graph Log	Sample format	Blows (N)	%Moisture water level	Fracture Density, Drill Notes, Gen Comment
0-4	1/20/06	Med-DK. Brown	LOAM SOIL. Approx. 40% SILT, 30% SAND, 20% CLAY AND 10% ORGANIC.		Grab, REVERSE Flow Discharge		Unknown	MUD 9/3 MIX, Baroid 18" BIT, Washed Sample 8 lbs. MUD (DRILLER)
4-7	1/20/06	DK. Brown	SILTY Clay. Approx 40% SILT, 50% clay 10% SAND. SOFT		Grab, REVERSE Flow Discharge		Unknown	Washed Sample
7-11	1/23/06	Tannish-gray	SILTY SAND. Approx. 25% SILT AND 80% V. FINE TO COARSE SAND. GRANITIC.		Grab, REVERSE Flow Discharge		Unknown	Washed Sample
11-13	1/23/06	Tannish-gray	SANDY GRAVEL. Approx. 60% FINE - V. COARSE SAND, 40% FINE - COARSE GRAVEL AND 10% COBBLE. ANGULAR TO WELL ROUNDED.		Grab, REVERSE Flow Discharge		Unknown	DIFFICULT DRILL, BIT BOUNCE, Washed Sample, Boring Enlargement, CABLE SPINNING
13-18	1/23/06	Tannish-gray	SANDY GRAVEL. Approx. 30% Med - V. COARSE SAND, 40% F. - C. GRAVEL, 20% COBBLE, Well ROUNDED. GRANITIC.		Grab, REVERSE Flow Discharge		Unknown	DRILL BIT BOUNCE, Slow DRILLING, Washed Sample, Boring Enlargement.
18-28	1/24/06	Tannish-gray	SANDY GRAVEL. Approx. 60% Med - V. COARSE SAND, 40% F. - C. GRAVEL, MINOR COBBLE. Well ROUNDED. GRANITIC.		Grab, REVERSE Flow Discharge		Unknown	Washed Sample Boring Enlargement.
28-43	1/24/06	TAN	SANDY SILTY CLAY. Approx. 10% FINE SAND, 30% SILT AND 60% CLAY. STICKY, Good Ribbon.		Grab, REVERSE Flow Discharge		Unknown	Washed Sample
43-60	1/25/06	Tan	SILTY CLAY. Approx. 30% SILT AND 70% CLAY. STICKY, Good Ribbon.		Grab, REVERSE Flow Discharge		Unknown	20" BIT AT 53 FT, Washed Sample
60-63	1/26/06	Tan	CLAYEY SANDY SILT. Approx. 15% CLAY, 20% FINE SAND AND 65% CLAY. STICKY Good Ribbon.		Grab, REVERSE Flow Discharge		Unknown	Washed Sample
63-64	1/26/06	Orangeish-Tan	SILTY CLAY. Approx. 30% SILT AND 70% CLAY. STICKY, Good Ribbon.		Grab, REVERSE Flow Discharge		Unknown	Washed Sample

# GEOTECHNICAL BOREHOLE LOG

HOLLADAY ENGINEERING COMPANY

BOREHOLE ID: EAGLE TEST Well No. 1 PROJECT NO.: EG 061204 PAGE: 2 of 6

PROJECT NAME: CITY OF EAGLE LOCATION: QUARTER CIRCLE D.I. RANCH, EAGLE, ID SITE LOCATION: SE, NW, S. 11, T. 4N, R. 1W

COLLAR EL: WR 63-32089 NORTHING: \_\_\_\_\_ EASTING: \_\_\_\_\_

HOLE DIAMETER: 18" 20" (BIT) DRILL METHOD: REVERSE ROTARY MUD DRILL MODEL: JED-A

DRILLER: RIVERSIDE, INC. LOGGER: G.D. + RIVERSIDE, INC. (JEFF HARR) STATIC WATER DEPTH: \_\_\_\_\_

Depth (ft)	Date Time	Induration & Color	Soil or Rock Description	Graph Log	Sample format	Blows (N)	%Moisture water level	Fracture Density, Drill Notes, Gen Comment
64-72	1/24/06	TAN	SILTY CLAY. APPROX. 30% SILT AND 70% CLAY. STICKY & GOOD RIBBON.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE
72-74	1/27/06	TANNISH-gray	SILTY SAND. APPROX. 30% SILT AND 70% FINE-MED. SAND. POORLY SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE, MUD TESTED AT 9 lbs. MIX.
74-92	1/27/06	TANNISH-gray	SAND. FINE TO COARSE GRAINED. WELL SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE
92-94	1/27/06	TAN	SILTY CLAY. APPROX. 60% SILT AND 40% CLAY. STICKY. MOD. RIBBON.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE
94-102	1/27/06	TANNISH-gray	SAND, FINE TO COARSE GRAINED. WELL SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE
102-104	1/27/06	TAN	SILTY CLAY. APPROX. 60% SILT AND 40% CLAY. MOD. RIBBON. STICKY.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE
104-109	1/27/06	TAN	SILTY CLAY. APPROX. 40% SILT AND 60% CLAY. GOOD RIBBON. STICKY.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE.
09-111	1/27/06	TAN	CLAYEY SILT. APPROX. 20% CLAY AND 80% SILT. POOR - NO RIBBON.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE.
11-116	1/27/06	TANNISH-gray	SAND. FINE - V. COARSE GRAINED. GRANITIC.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE.
116-123	1/27/06	TAN	CLAY. APPROX. 20% SILT AND 80% CLAY. STICKY. GOOD RIBBON.		Grab, REVERSE Flow Discharge		Unknown	WASHED SAMPLE.

# GEO. TECHNICAL BOREHOLE LOG

## HOLLADAY ENGINEERING COMPANY

BOREHOLE ID: EAGLE TEST WELL No. 1 PROJECT NO.: EG-06-1204 PAGE: 3 of 6

PROJECT NAME: CITY OF EAGLE LOCATION: QUARTER CIRCLE D.T. SITE LOCATION: SE, NW, S. 11, T. 4N, R. 1W  
WR 63-32089  
WR 63-32090 RAUCH

COLLAR EL: \_\_\_\_\_ NORTHING: \_\_\_\_\_ EASTING: \_\_\_\_\_

HOLE DIAMETER: 18", 20" (BIT) DRILL METHOD: REVERSE ROTARY MUD DRILL MODEL: JED-A

DRILLER: RIVERSIDE, INC. LOGGER: C.D. & RIVERSIDE, INC. STATIC WATER DEPTH: \_\_\_\_\_  
(JEFF HARRIS)

Depth (ft)	Date Time	Induration & Color	Soil or Rock Description	Graph Log	Sample format	Blows (N)	%Moisture water level	Fracture Density, Drill Notes, Gen Comment
123-126	1/27/06	TANNISH-GRAY	SAND, FINE TO V. COARSE SAND. WELL SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
126-131	1/27/06	Tan	SILTY CLAY. APPROX. 50% SILT AND 50% CLAY. STICKY. GOOD RIBBON.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
131-134	1/27/06	TANNISH-GRAY	SAND, FINE TO COARSE GRAINED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
134-140	1/27/06	Tan	SILTY CLAY. APPROX. 60% SILT AND 40% CLAY. MOD. RIBBON. STICKY.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
140-157	1/27/06	TANNISH-GRAY	SAND, FINE - COARSE GRAINED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
157-161	1/28/06	Tan	SILTY CLAY. APPROX. 50% SILT AND 50% CLAY. SOFT, STICKY. GOOD RIBBON.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
161-166	1/28/06	TANISH-GRAY	SAND, FINE TO COARSE WELL SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
166-174	1/28/06	Tan	CLAY, APPROX. 30% AND 70% CLAY. SOFT, STICKY. GOOD RIBBON.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
174-181	1/28/06	Tan	SANDY SILTY CLAY. APPROX. 10% FINE SAND, 30% SILT AND 60% CLAY. SOFT, STICKY. MOD. RIBBON.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
181-191	1/28/06	TANISH-GRAY	SILTY CLAY AND SAND. APPROX. 10% CLAY, 30% SILT AND 60% SAND. V. FINE TO COARSE. POORLY SORTED.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.

# GEO TECHNICAL BOREHOLE LOG

## HOLLADAY ENGINEERING COMPANY

BOREHOLE ID: EAGLE TEST WELL No. 1 PROJECT NO.: EG 061204 PAGE: 4 of 6

PROJECT NAME: CITY OF EAGLE LOCATION: QUARTER CIRCLE D.T. RANCH SITE LOCATION: SE, NW, S. 11, T. 4N. R. 1W  
 WR 63-32089  
 WR 63-32090

COLLAR EL: \_\_\_\_\_ NORTHING: \_\_\_\_\_ EASTING: \_\_\_\_\_

HOLE DIAMETER: 18", 20" DRILL METHOD: REVERSE ROTARY MUD DRILL MODEL: JED-A  
 (B.T.)

DRILLER: RIVERSIDE, INC. LOGGER: C.D. & RIVERSIDE JLC. STATIC WATER DEPTH: \_\_\_\_\_  
 (SFF HASH)

Depth (ft)	Date Time	Induration & Color	Soil or Rock Description	Graph Log	Sample format	Blows (N)	%Moisture water level	Fracture Density, Drill Notes, Gen Comment
191-203	1/28/06	TANNISH-GRAY	SAND, FINE TO COARSE GRAINED. MOD. SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
203-221	1/28/06	TANNISH-GRAY	CLAYEY SILTY SAND. APPROX. 10% CLAY, 20% SILT AND 70% U. FINE TO MED. SAND. POORLY SORTED.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
221-223	1/28/06	TAN	SILTY CLAY. APPROX. 80% SILT AND 20% CLAY. MOD. RIBBON		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. NOTE: REPORT AS STRINGERS, BASED ON DRILL RATE.
223-257	1/28/06	TANNISH-GRAY	SAND, FINE TO MED. GRAINED SAND. WELL SORTED. SUB ANGULAR. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. NOTE: CLAY STRINGERS AT 234 FT. REPORTED BY DRILLER.
257-264	1/28/06	TAN	CLAY. APPROX. 10% SILT AND 90% CLAY. SOFT, STICKY. GOOD RIBBON.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. NOTE: SAND STRINGER REMOVED BY DRILLER AT 260 FT.
264-320	1/28/06	TANNISH-GRAY	SAND. V. FINE TO MED. GRAINED. WELL SORTED. SUB ANGULAR. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. NOTE: CLAY STRINGER REPORTED BY DRILLER AT 292 FT. AND 313 FT.
320-353	1/30/06	TANNISH-GRAY	SAND GRADING TO SILTY SAND. APPROX. 20% SILT AND 80% V. FINE TO MED. SAND. MODERATELY SORTED.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
353-370	1/30/06	BLUE-GRAY	SANDY SILTY CLAY. APPROX. 20% V. FINE SAND, 20% SILT AND 60% CLAY. SOFT. POORLY SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
370-377	1/30/06	TANNISH-GRAY	SILTY SAND. APPROX. 10% CLAY, 20% SILT AND 70% V. FINE TO COARSE SAND. POORLY SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. 9.0 lbs MUD (DRILLER)
377-384	1/30/06	BLUE-GRAY	SANDY SILTY CLAY. APPROX. 20% V. FINE TO FINE SAND, 20% SILT AND 60% CLAY. SOFT, STICKY MOD. RIBBON. 5% CARBON WOOD.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.

# GEOTECHNICAL BOREHOLE LOG

## HOLLADAY ENGINEERING COMPANY

BOREHOLE ID: EAGLE TEST WELL No. 1 PROJECT NO.: EG 061204 PAGE: 5 of 6

PROJECT NAME: CITY OF EAGLE LOCATION: QUARTER CIRCLE D.T. RANCH SITE LOCATION: SE, NW, S. 11, T. 4N, R. 1W  
 WR 63-32089  
 WR 63-32090

COLLAR EL: \_\_\_\_\_ NORTHING: \_\_\_\_\_ EASTING: \_\_\_\_\_

HOLE DIAMETER: 18" 20" DRILL METHOD: REVERSE ROTARY MUD DRILL MODEL: JED-A  
 (BIT)

DRILLER: RIVERSIDE, INC. LOGGER: C.D. & RIVERSIDE INC. STATIC WATER DEPTH: \_\_\_\_\_  
 (JEFF NASH)

Depth (ft)	Date Time	Induration & Color	Soil or Rock Description	Graph Log	Sample format	Blows (N)	%Moisture water level	Fracture Density, Drill Notes, Gen Comment
384-387	11/30/06	Tanish-gray	SAND. Approx. 90% V. FINE TO MED. SAND AND 10% SILT. WELL SORTED. GRANITIC,		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
387-419	11/30/06	Blue-gray	CLAY. Approx 80% CLAY AND 20% SILT. SOFT. Good Ribbon.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
419-434	11/30/06	LT. GRAY	SAND. Approx. 15% SILT AND V. FINE TO V. COARSE SAND, MINOR Pebbles. Med. SORTED.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
434-444	11/30/06	LT. GRAY	CLAYEY SILT SAND. Approx. 20% CLAY, 40% SILT AND 40% V. FINE TO MED. SAND. POORLY SORTED.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
444-459	11/30/06	Blue-gray	SILTY CLAY. Approx. 60% SILT AND 40% CLAY. SOFT. Med. Ribbon.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
459-465	11/30/06	GRAY	CLAYEY SILTY SAND. Approx. 10% CLAY, 20% SILT AND 70% V. FINE TO MED. SAND. POORLY SORTED.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. STOPPED DRILLING by Engineer.
465-482	11/31/06	LT. GRAY	SAND, FINE TO MED. SAND. WELL SORTED. GRANITIC, CLAY STRING 6" AT 475'. CARBONIZED Wood (475-480').		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. RESUMED DRILLING by Engineer. MUD 9.7 lbs (DRILLED)
481-482	11/31/06	BLUE-GRAY	SILTY CLAY. Approx. 60% SILT AND 40% CLAY. SOFT. Good Ribbon.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
482-483	11/31/06	LT. GRAY	CLAYEY SILT SAND. Approx. 10% CLAY, 30% SILT AND 60% FINE TO COARSE SAND POORLY SORTED. CARBONIZED Wood Frag.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
483-484	11/31/06	Blue-gray	SILTY CLAY. Approx. 40% SILT AND 60% CLAY. 5-10% CARBONIZED Wood Frag.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.

# GEO TECHNICAL BOREHOLE LOG

## HOLLADAY ENGINEERING COMPANY

BOREHOLE ID: EAGLE TEST WELL No. 1 PROJECT NO.: EG 061204 PAGE: 6 of 6

PROJECT NAME: CITY OF EAGLE LOCATION: QUARTER CIRCLE D.I. RANCH SITE LOCATION: SE NW, S. 11, T. 4N, R. 1W

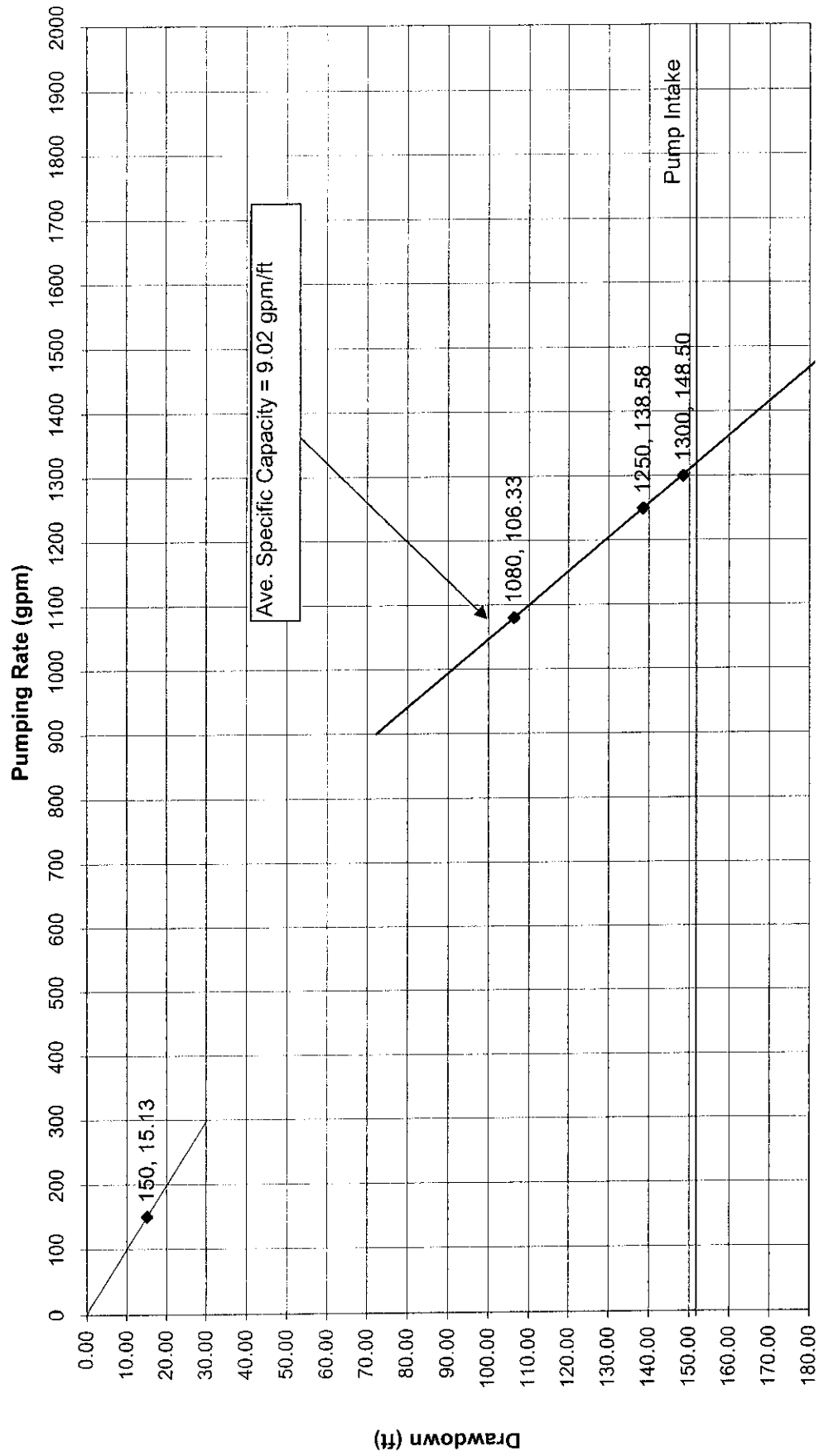
COLLAR EL: WR 63-32089 WR 63-32090 NORTHING: \_\_\_\_\_ EASTING: \_\_\_\_\_

HOLE DIAMETER: 18" 20" (BIT) DRILL METHOD: REVERSE ROTARY MUD DRILL MODEL: JED-A

DRILLER: RIVERSIDE, INC. LOGGER: C.D. & RIVERSIDE INC. (JEFF WASH) STATIC WATER DEPTH: \_\_\_\_\_

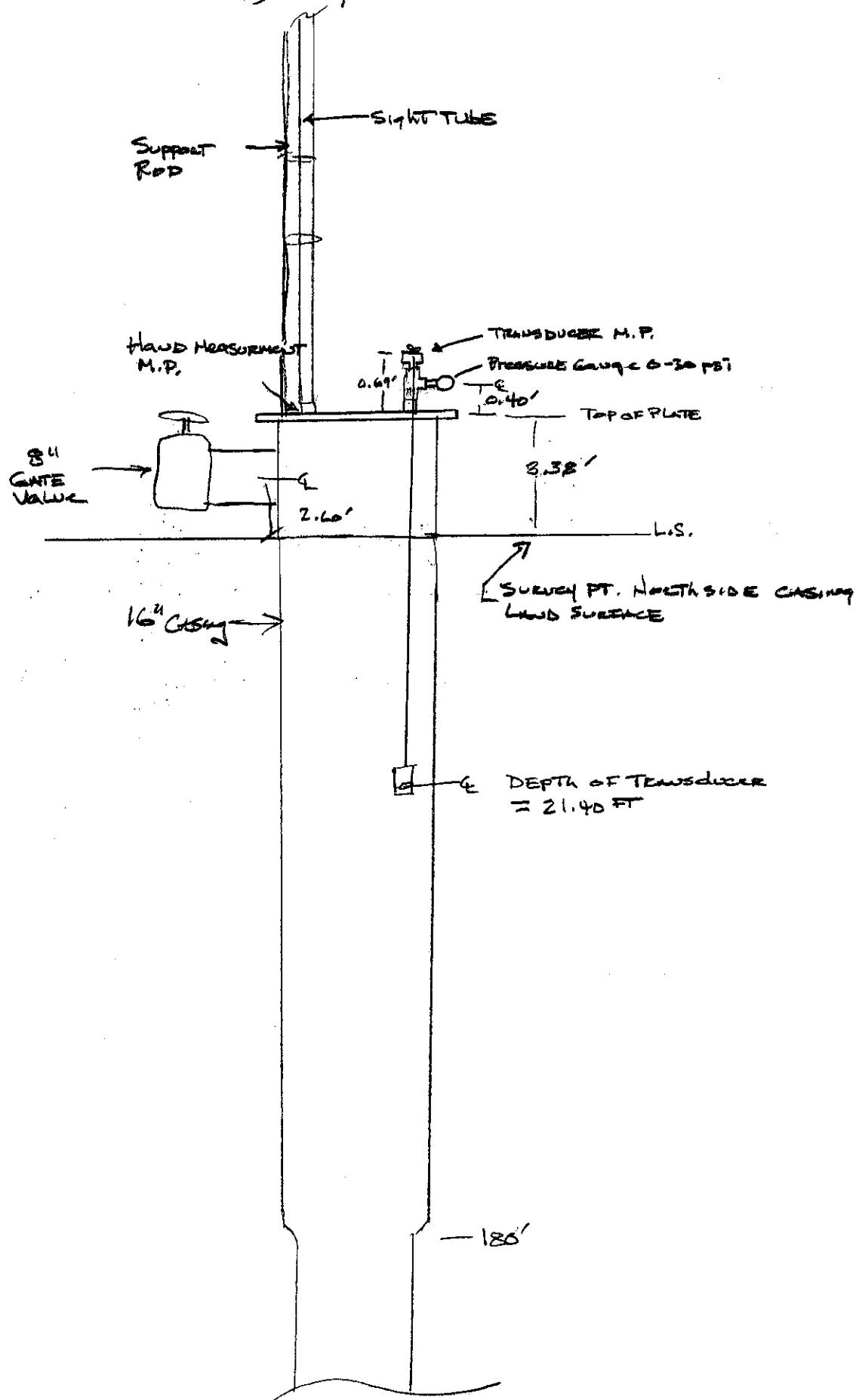
Depth (ft)	Date Time	Induration & Color	Soil or Rock Description	Graph Log	Sample format	Blows (N)	%Moisture water level	Fracture Density, Drill Notes, Gen Comment
484-486	11/31/06	LT. GRAY	SAND: FINE-MED, SAND, WELL SORTED, WEAKLY CEMENTED. CARBONITE? WELL SORTED, GRANITIC, DRY, SED. FRAG. CARBONIZED WOOD. FRAG.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. HARDER DRILLING.
486-487	11/31/06	BLUE-GRAY	CLAY, SOFT. STICKY. Good Ribbon.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
487-489	11/31/06	LT. GRAY	SAND, MED. TO COARSE SAND AND MINOR FINE SAND. WELL SORTED. GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
489-493	11/31/06	LT. GRAY	SAND, MED. TO V. COARSE SAND AND 10% PEBBLES. SUBANGULAR, MOD. SORTED, GRANITIC.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE.
493-501	11/31/06	DK. GRAY TO BLUE-GRAY	CLAY. STIFF. Good Ribbon. GRADING IN COLOR FROM DK. GRAY TO BLUE-GRAY.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. DRILL RIG BOUNCE.
501-505	11/31/06	Weak BLUE-GRAY TO GRAY	CLAYEY SILTY SAND. APPROX. 10% CLAY, 90% SILT, AND V. FINE TO FINE SAND. POORLY SORTED. WEAKLY CEMENTED CARBONITE.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. HARDER DRILLING.
505-513	11/31/06	BLUE-GRAY	CLAY, MOD. STIFF. STICKY. Good Ribbon.		Grab, REVERSE Flow Discharge		UNKNOWN	WASHED SAMPLE. T.D. 513' ENGINEER STOPPED DRILLING.

**STEP PUMP TEST of TEST WELL No.1 (Legacy)**  
**March 24th 9:15am to 2:30 pm**  
**Specific Capacity**



EAGLE 7-DAY 1 WATER TEST

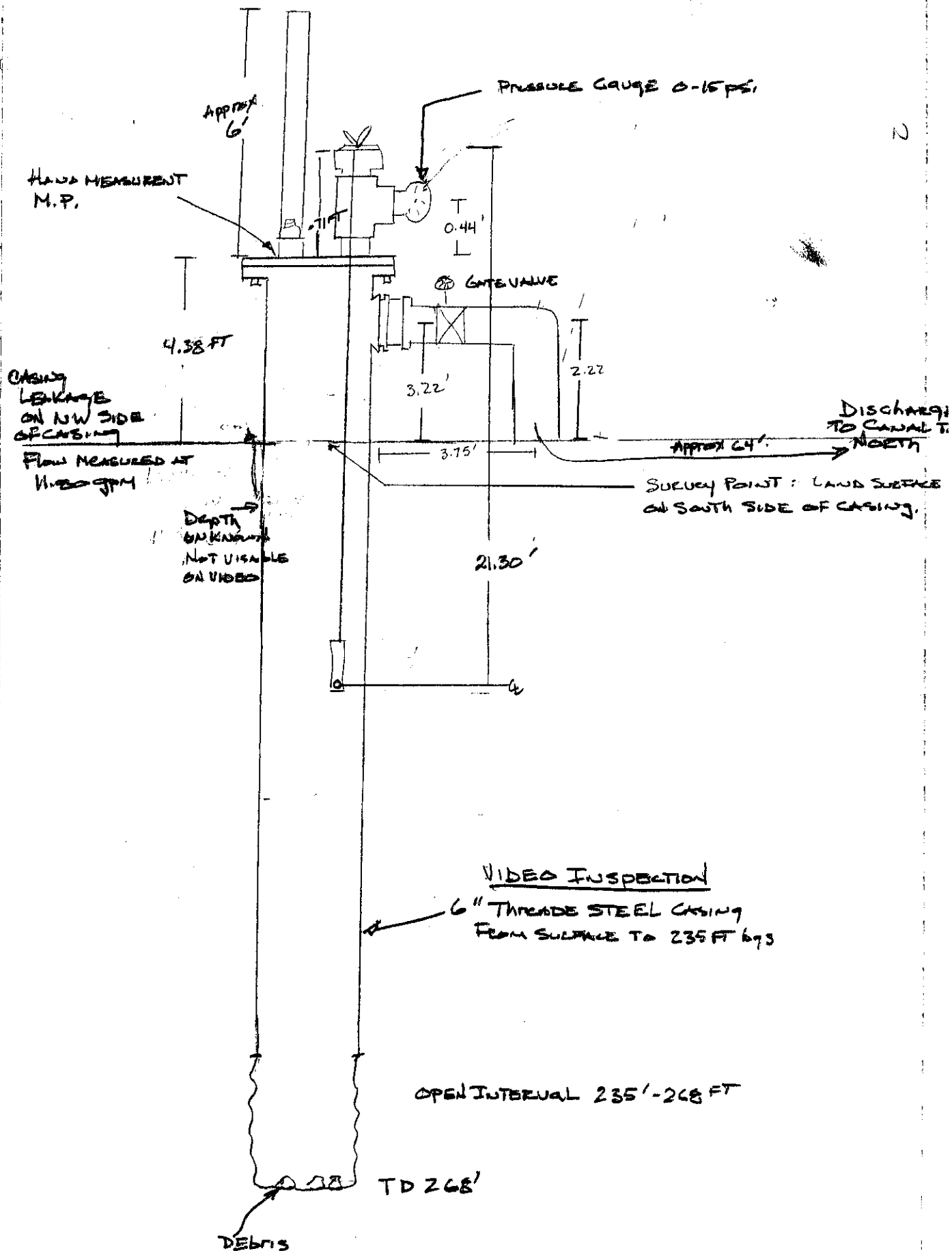
TEST WELL No. 2 (LEGACY) ↑ 18+'



## Appendix B.3

### MONITORING WELL 4 (QCR 4)

QUARTER CIRCLE WELL NO. 4  
MONITORING WELL NO. 4 (QCR4)



## Appendix B.4

### MONITORING WELL 6 (Rick's)

## WELL DRILLER'S REPORT

State law requires that this report be filed with the Director, Department of Water Administration within 30 days after the completion or abandonment of the well.

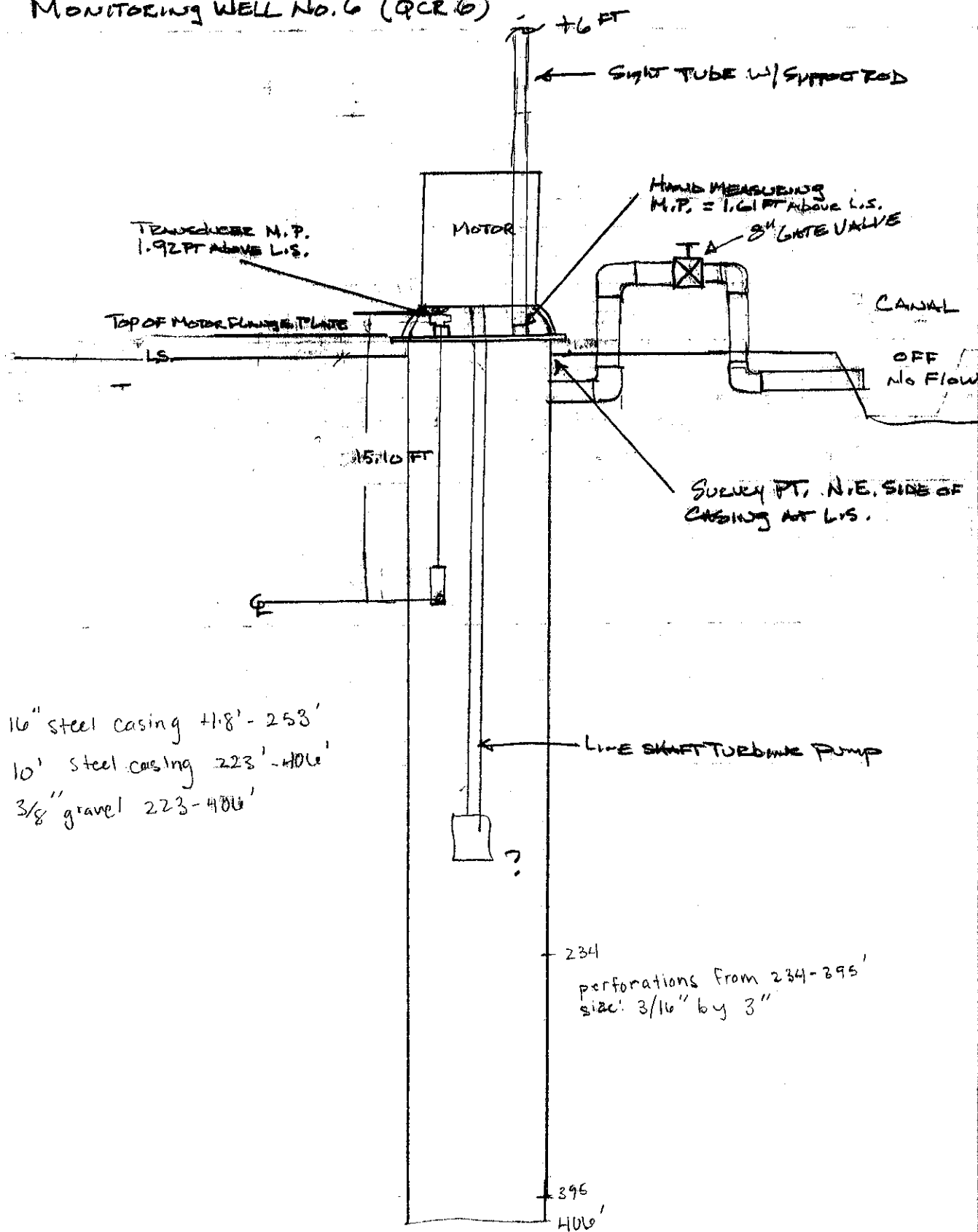
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CITY OF EAGLE 1-DAY AQUIFER TEST

QUARTER CIRCLE WELL No. 6

(Rick's Irrigation Well)

MONITORING WELL No. 6 (QCR 6)




## **Appendix B.5**

### **MONITORING WELL 9 (Strata 1A)**

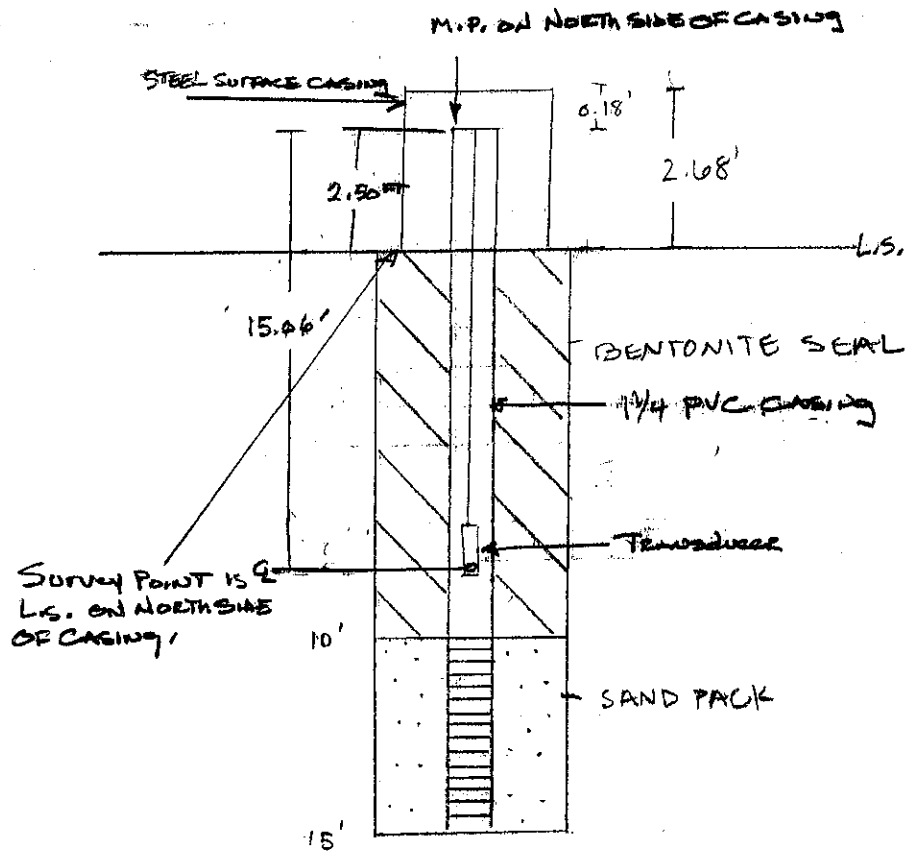
Draft

Boring No. 1a												REMARKS	
Subsurface Soil Description		DEPTH (In Feet)	USCS CLASS	SYMBOL	SAMPLE Type	BLOWS Per 6 inches	SPT (Corrected) Blows Per Foot	POCKET Penetro- meter (tsf)	WELL CONSTRUCTION			Note: BGS = Below Ground Surface	
Sandy SILT (Native) - tan, very stiff, moist.		0	ML									Trace vegetation and organics observed to 3 inches BGS.	
CLAY with SAND - brown, soft to stiff, moist to saturated.		1	CL									Top of protective steel casing above ground = 2.8 feet.	
		2											
		3										Bentonite Seal from 0 to 10 feet	
		4										1 1/4 inch Ø PVC Well	
		5											
Poorly-Graded GRAVEL with Sand and Cobbles - light brown, dense, saturated.		6	GP										
		7	*										
		8											
*Reading on 9-7-2005 = 7.1 feet		9											
		10											
		11										Sand pack from 10 to 15 feet	
		12										1 1/4 inch Ø PVC screened well from 10 to 15 feet	
		13											
		14											
Boring terminated at 15 feet BGS.		15										Standpipe piezometer installed to 15 feet.	
		16											
		17											
		18											
		19											
		20											

File: EAGSPO	Boring Number: B-1a	 GEOTECHNICAL ENGINEERING & MATERIALS TESTING Integrity from the Ground Up	EXPLORATORY BORING LOGS
Project No.: B05188A	Date Drilled: 8-30-2005		
Drill Rig: BK-81	Boring Diameter: 8 inch		
Depth to Groundwater: 6.5'	Logged By: AM		
			Sheet 1 of 1

F:\Projects\ExpSps\B05188a\deg\B05188a-01\_1-2.dwg 9/19/2005 6:13:07 PM MDT

CITY OF EAGLE 7-DAY AQUIFER  
MONITORING WELL NO. 9 (STRATA 1A)



## **Appendix B.6**

### **MONITORING WELL 10 (Strata 1B)**


2/2/2005

Draft

Boring No. 1		DEPTH (In Feet)	USCS CLASS	SYMBOL	SAMPLE Type	BLOWS Per 6 inches	SPT (Corrected) Blows	POCKET Penetro- meter (tsf)	WELL CONSTRUCTION	REMARKS
Subsurface Soil Description Top of Casing Elevation = Ground Surface Elevation =										Note: BGS = Below Ground Surface
Sandy SILT (Native) - tan, very stiff, moist.		1	ML							Trace vegetation and organics observed to 3 inches BGS.  Top of protective steel casing above ground = 3.1 feet.  Passing #200 screen = 78%.  BG  Liquid Limit (LL) = 30. Plastic Index (PI) = 11. Moisture Content = 24.9%.  BG
CLAY with SAND - brown, soft to stiff, moist to saturated.		2	CL							
		3				1				
		4				1		0.75		
		5				2				
		6				6		1.0		
		6				10	16	1.25		BG
Poorly-Graded GRAVEL with Sand and Cobbles - light brown, dense, saturated.		7	GP							BG  BG  Bentonite Seal 1 inch Ø PVC Well  Heaved sand from 15 to 16 feet.  BG BG
		7	*							
		8				32				
		9				25				
		10				38				
		11								
		12								
		13								
		14								
		15								
		15								
		16				4				
		16				15	26			BG
		17				27				BG
		18								
		19								
		20								

\*Reading on 9-7-2005 =  
6.8 feet

File: EAGSP0	Boring Number: B-1
Project No.: B05188A	Date Drilled: 8-30-2005
Drill Rig: BK-B1	Boring Diameter: 8 inch
Depth to Groundwater: 6'	Logged By: AM



**STRATA**  
GEOTECHNICAL ENGINEERING & MATERIALS TESTING  
*Integrity from the Ground Up*

**EXPLORATORY  
BORING LOGS**

Sheet 1 of 3

F:\Projects\Boring\B05188A\Boring\B05188A-bl 1-2.dwg 9/19/2005 5:16:00 PM MDT

*Draft*

Boring No. 1 Subsurface Soil Description	DEPTH (in Feet)	USCS CLASS	SYMBOL	SAMPLE Type	BLOWS Per 6 Inches	SPT (Corrected) Blows	Per Foot Blows	POCKET Penetro- meter (tsf)	WELL CONSTRUCTION	REMARKS  Note: BGS = Below Ground Surface
Poorly-Graded GRAVEL with Sand and Cobbles - light brown, dense, saturated.	21	GP			7 39 32	43				<div>BG</div> Heaved sand from 20 to 21 feet.
	22									
	23									
Fat CLAY - dark gray, hard to very stiff, wet.	24	CL								
	25									
	26				26 34 42	61				<div>BG</div> Minimal recovery, possible rock at bottom of spoon.  Passing #200 screen = 96%.
	27									
	28				11 15 26	31		>4.5 4.0 3.5		<div>BG</div> <div>RG</div> <div>RG</div> Liquid Limit (LL) = 58. Plastic Index (PI) = 31.
	29									
	30				10 13 20	33		3.0 3.5		<div>BG</div>
	31									
	32									
	33									Bentonite Seal 1 inch Ø PVC Well
	34									
	35				7 12 17	29		1.5 3.0		<div>BG</div>
...except light brown at 36.5 feet.	36									
	37									
Clayey SAND - light brown, dense, saturated.	38	SC								
	39									
	40									

File: EAGSPO	Boring Number: B-1
Project No.: B05188A	Date Drilled: 8-30-2005
Drill Rig: BK-81	Boring Diameter: 8 inch
Depth to Groundwater: 6'	Logged By: AM



**EXPLORATORY  
BORING LOGS**

Sheet 2 of 3

F:\Projects\EagSp\B05188A\B05188A.dwg 9/19/2005 5:56:17 PM KDT

Draft

Boring No. 1 Subsurface Soil Description	DEPTH (in Feet)	USCS CLASS	SYMBOL	SAMPLE Type	BLOWS Per 6 inches	SPT (Corrected) Blows	POCKET Penetro- meter (tsf)	WELL CONSTRUCTION	REMARKS Note: BGS = Below Ground Surface
Clayey SAND - light brown, dense, saturated.	41	SC			27 30 41	47			Trace orange staining observed in sample. BG RG RG Bentonite Seal from 0 to 45 feet 1 inch $\phi$ PVC Well
Poorly-Graded medium SAND - light brown, very dense, saturated.	45	SP			29 56 60	69			RG Sand pack from 45 to 55 feet 1 inch $\phi$ PVC screened well from 45 to 55 feet
	50				33 59 62	71			RG BG Standpipe piezometer installed to 55 feet.
Boring terminated at 55 feet BGS.	55								
	56								
	57								
	58								
	59								
	60								

F:\Projects\EngSpa\B05188a\dwg\B05188a-bl 1-2.dwg 9/19/2005 6:03:23 PM NDT

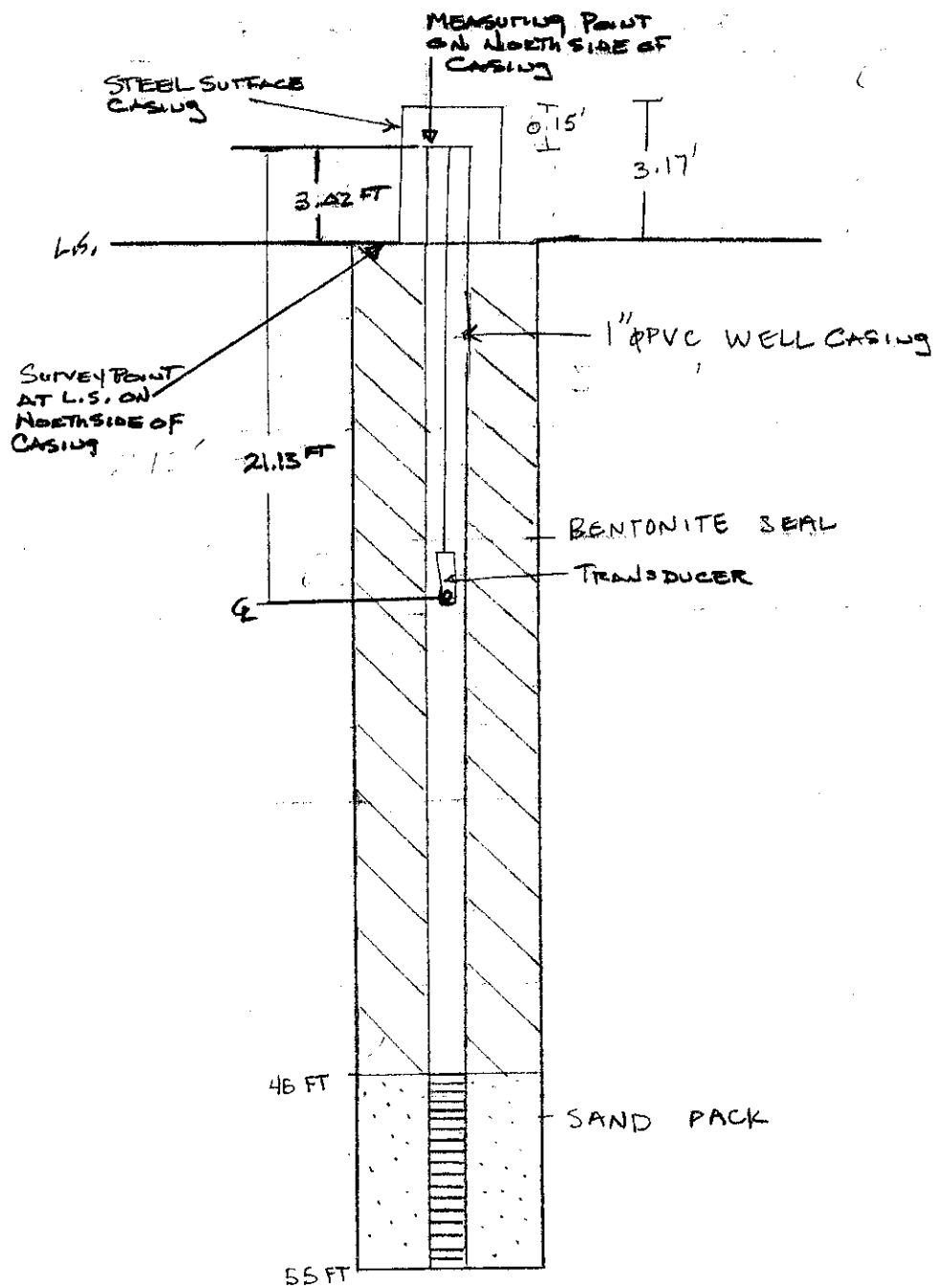
File: EAGSPO	Boring Number: B-1
Project No.: B05188A	Date Drilled: 8-30-2005
Drill Rig: BK-81	Boring Diameter: 8 inch
Depth to Groundwater: 6'	Logged By: AM



**EXPLORATORY  
BORING LOGS**

Sheet 3 of 3

CITY OF EAGLE 7<sup>th</sup> AQUIFER TEST  
MONITORING Well No. 10 (STRATA 1B)



## **Appendix B.7**

### **MONITORING WELL 11 (UWI 1A)**



Univ 2 Water State & Line e 63-97-W-0633-801  
test Well #1 67931

ground +1' —  
Level —  
A B 8" 250 wall casing Not to scale

101' — 8" Driv shoe

bentonite  
grout  
pumped up from  
230'

All Plastic is 2"  
sch. 80

280' —  
290' —  
300' —  
310' —  
320' —  
330' —  
340' —  
350' —  
360' —  
370' —  
380' —

Natural sand pack  
230' to 545'

Both Wells flow  
under 14 1/2' of head

— 400'  
— 410'  
— 420'  
— 430'  
— 440'  
— 450'  
— 460'  
— 470'  
— 480'  
— 490'  
— 500'

PROPOSED

AS 0.9 1987

— 545'

RECEIVED

OCT 23 1997

WATER RESOURCES  
WESTERN REGION

63-97-W-0633-801

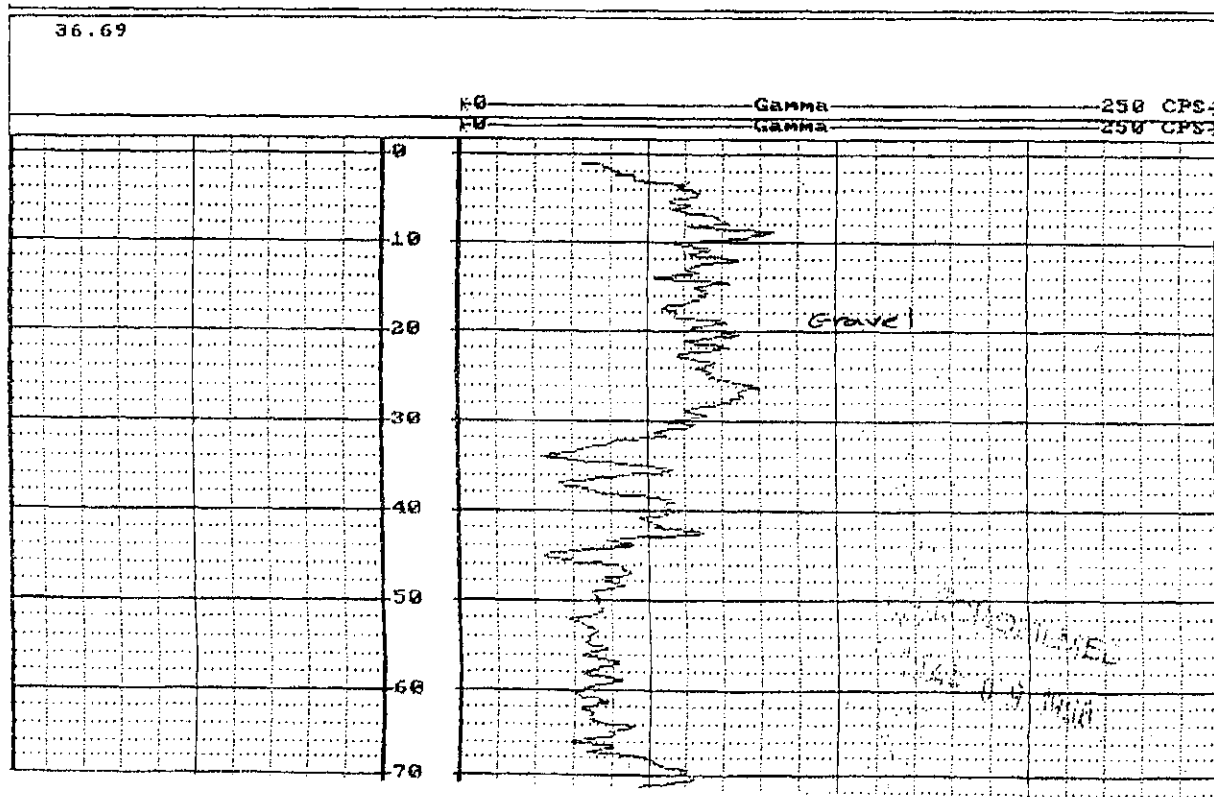
67932

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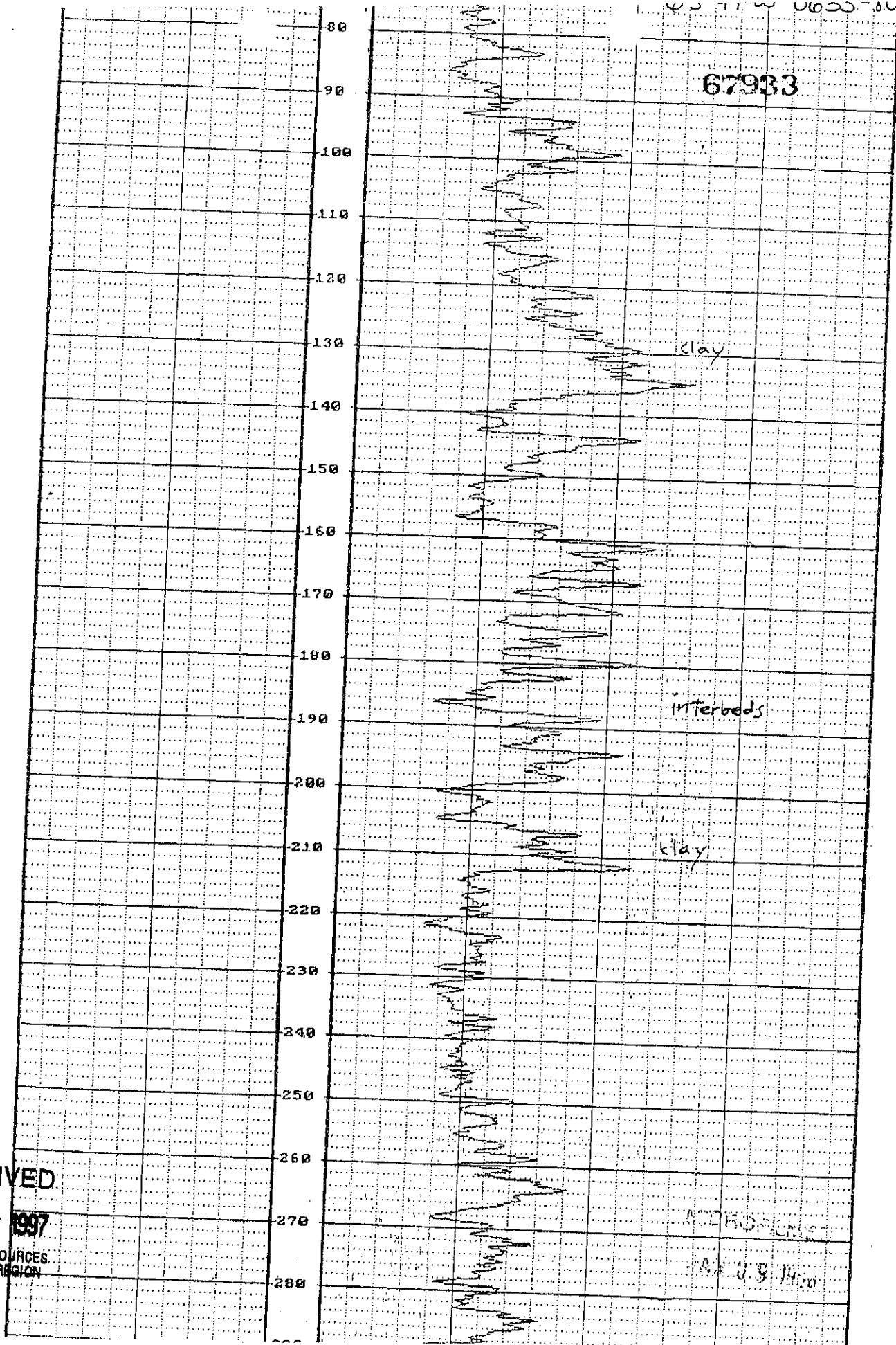
OCT 23 1997

WATER RESOURCES  
WESTERN REGION

State # Linder Test Well



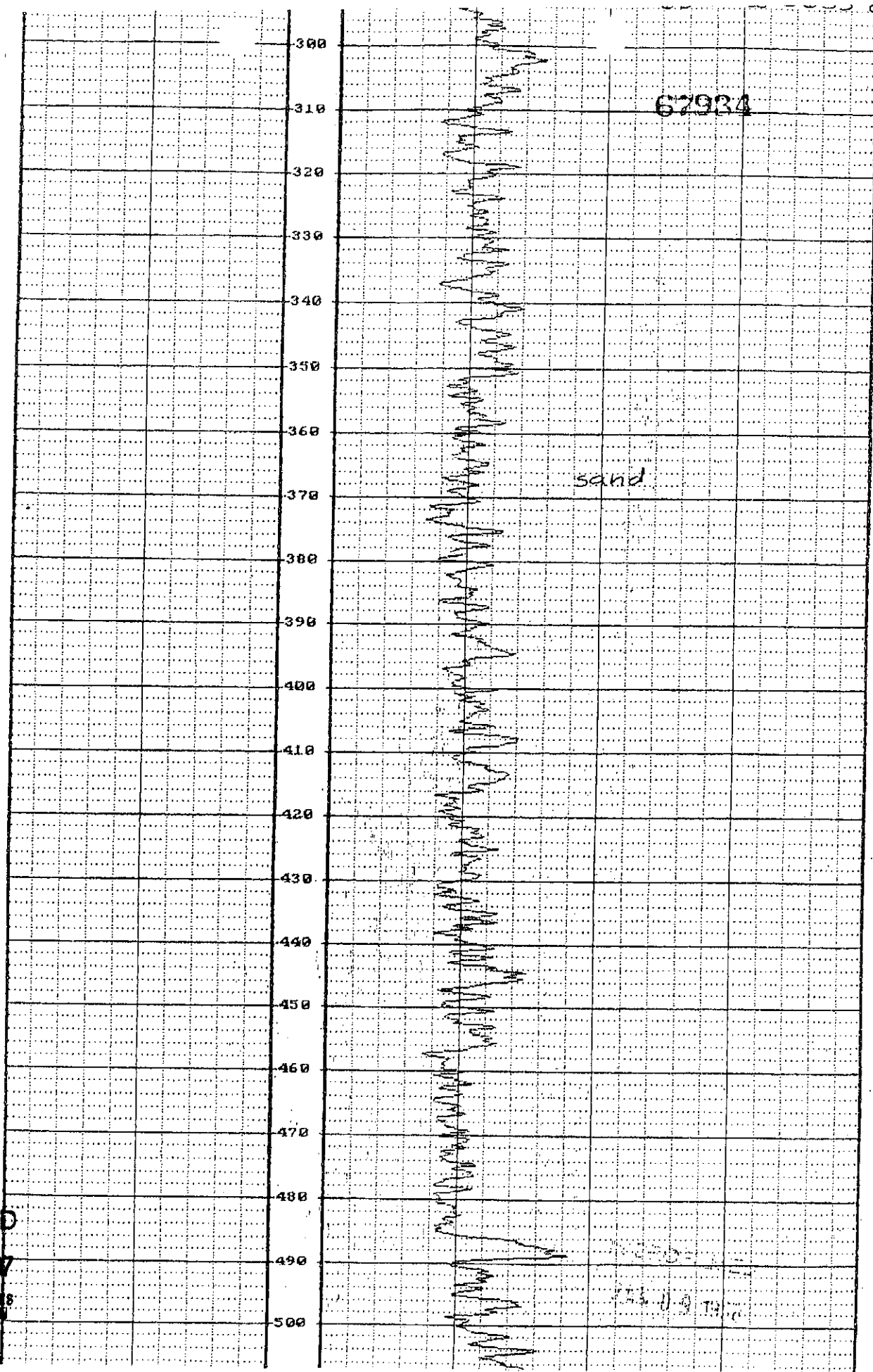
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RECEIVED

OCT 23 1997

WATER RESOURCES  
WESTERN REGION



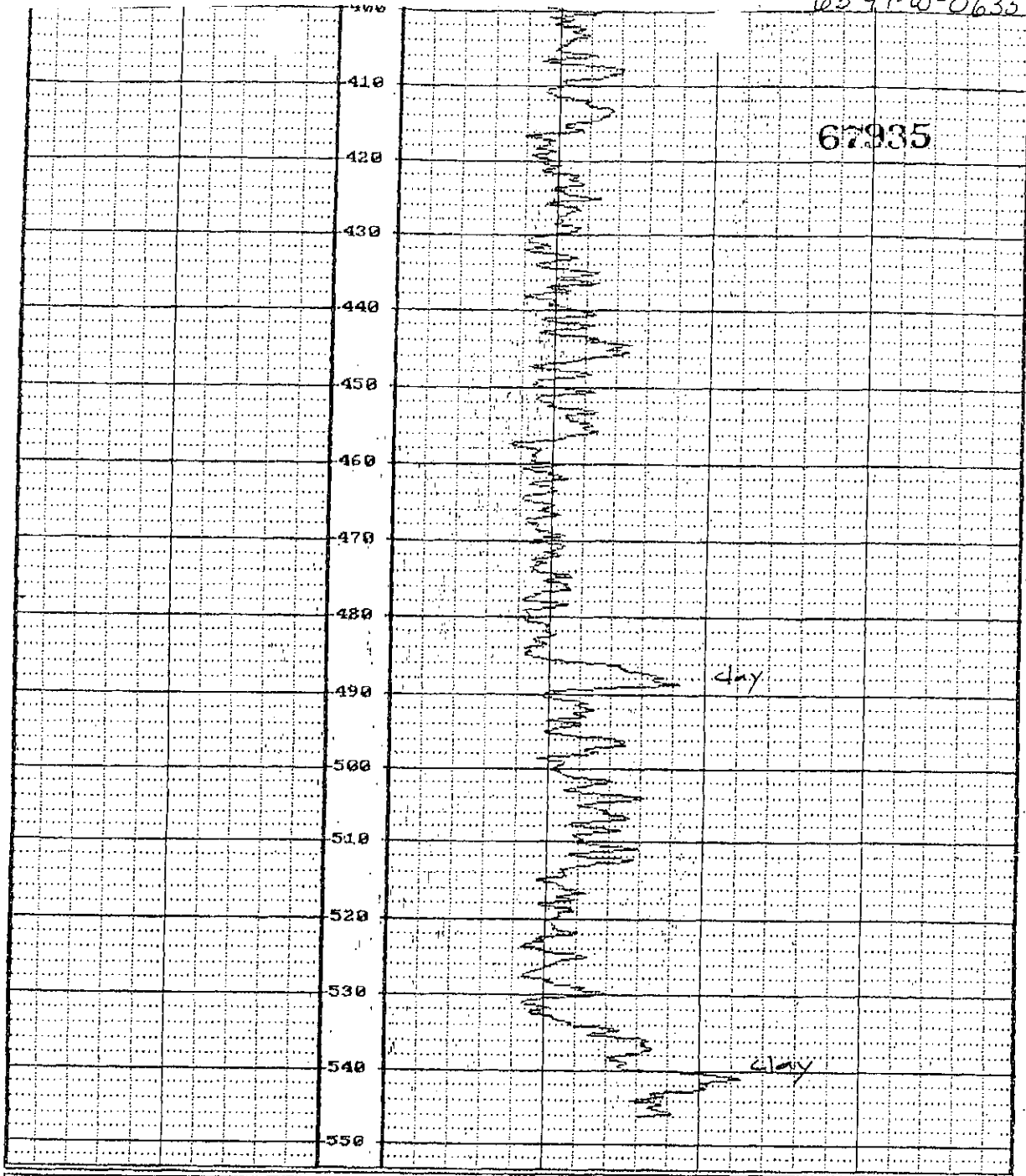
62934

sand

RECEIVED

ICT 23 1997

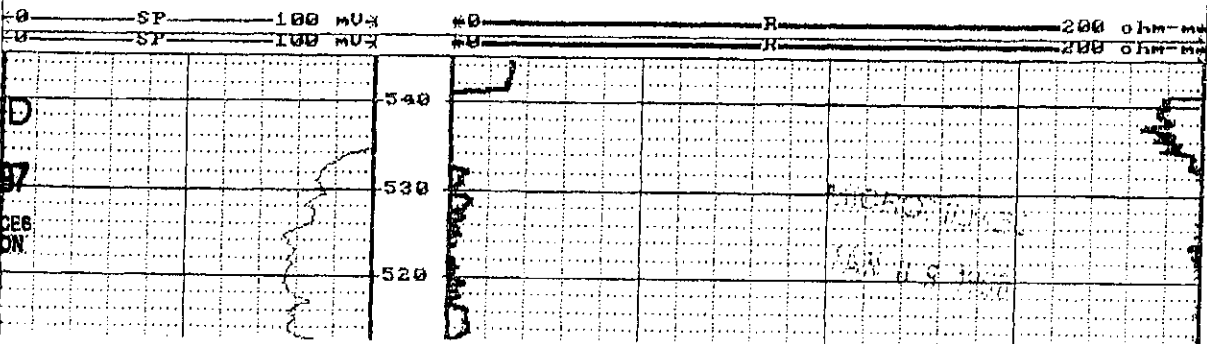
WATER RESOURCES  
V. N. ERN REGION



108.04

Gamma 250 CPS

351.25 215.16



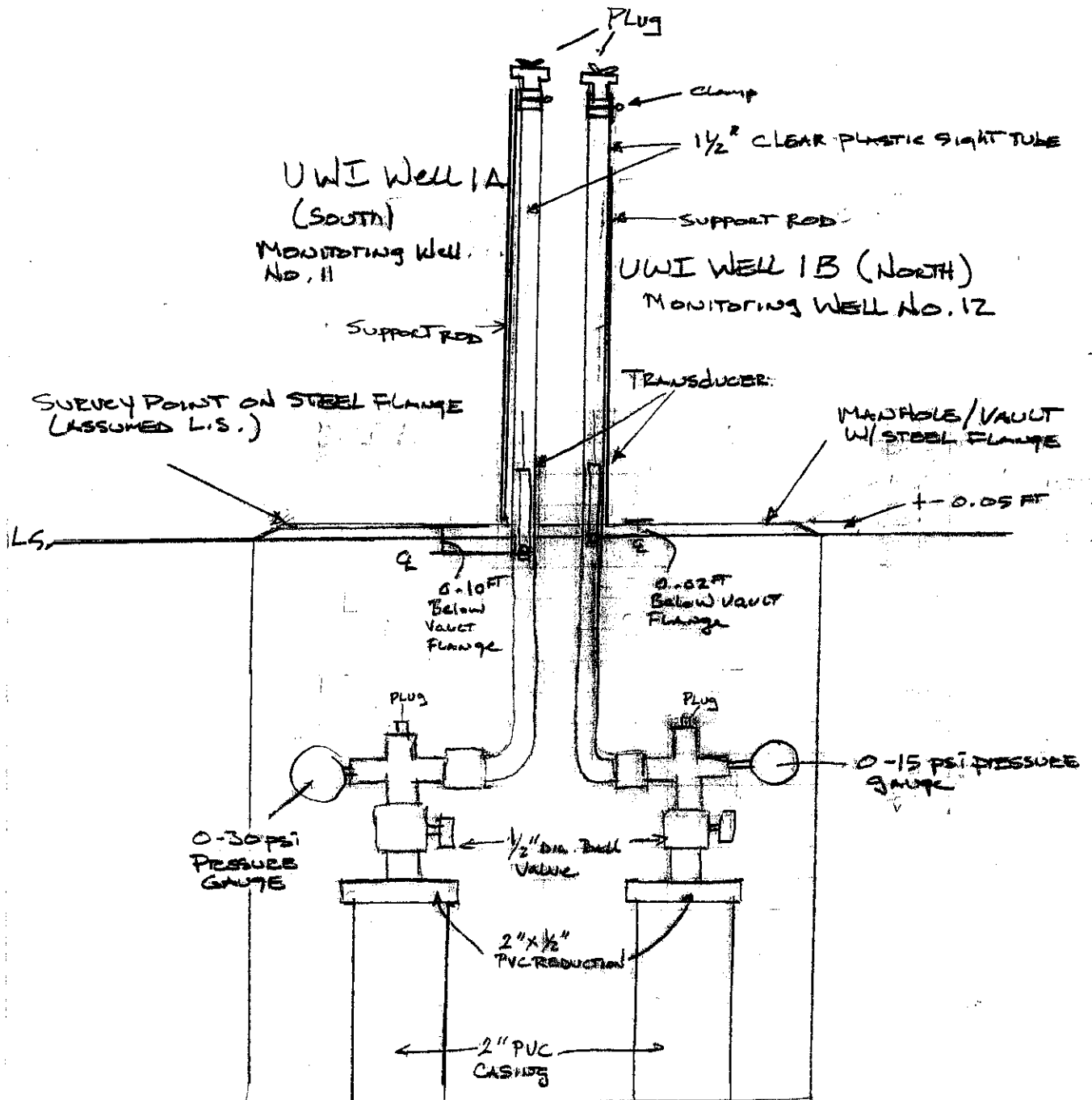
RECEIVED

OCT 23 1997

WATER RESOURCES  
WESTERN REGION

RECEIVED  
OCT 23 1997  
WATER RESOURCES  
WESTERN REGION

CITY OF EAGLE 1-DAY AQUIFER TEST  
MONITORING WELL NO. 11 (UWI 1A) AND  
MONITORING WELL NO. 12 (UWI 1B)



## **Appendix B.8**

### **MONITORING WELL 12 (UWI 1B)**



United Water State & Line R 63-97-W-0633-801  
test Well #1 67931

ground level +1' A B 8" 250 wall casing Not to scale

101' 8" Drive shoe

bentonite  
grout  
pumped up from  
230'

All Plastic is 2"  
sch. 80

280'  
290'  
300'  
310'  
320'  
330'  
340'  
350'  
360'  
370'  
380'

Natural sand pack  
230' to 545'

Both Wells flow  
under 14 1/2' of head

400'  
410'  
420'  
430'  
440'  
450'  
460'  
470'  
480'  
490'  
500'

CONTINUED

SEE 0.9 1997

545'

RECEIVED

OCT 23 1997

WATER RESOURCES  
WESTERN REGION

63-97-W-0633-80.

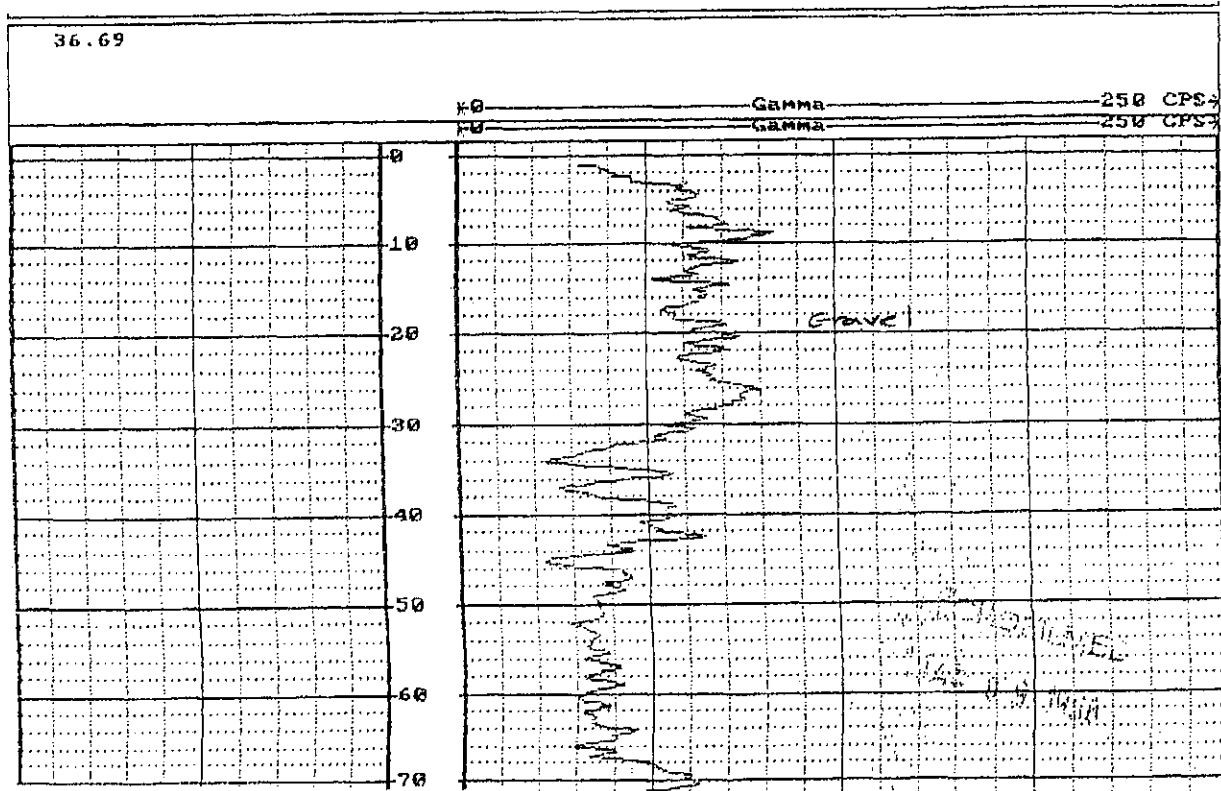
67932

RECEIVED

OCT 23 1997

WATER RESOURCES  
WESTERN REGION

State # Linder Test Well



67933

A vertical scale with numerical labels 80, 90, and 100 at the top, and 110, 120, 130, 140, 150, 160, 170, 180, 190, 200, 210, 220, 230, 240, 250, 260, 270, 280, 290, 300, 310, 320, 330, 340, 350, 360, 370, 380, 390, 400, 410, 420, 430, 440, 450, 460, 470, 480, 490, 500, 510, 520, 530, 540, 550, 560, 570, 580, 590, 600, 610, 620, 630, 640, 650, 660, 670, 680, 690, 700, 710, 720, 730, 740, 750, 760, 770, 780, 790, 800, 810, 820, 830, 840, 850, 860, 870, 880, 890, 900, 910, 920, 930, 940, 950, 960, 970, 980, 990, 1000 at the bottom. Horizontal grid lines extend from each label across the page.

May

interbeds

ctay

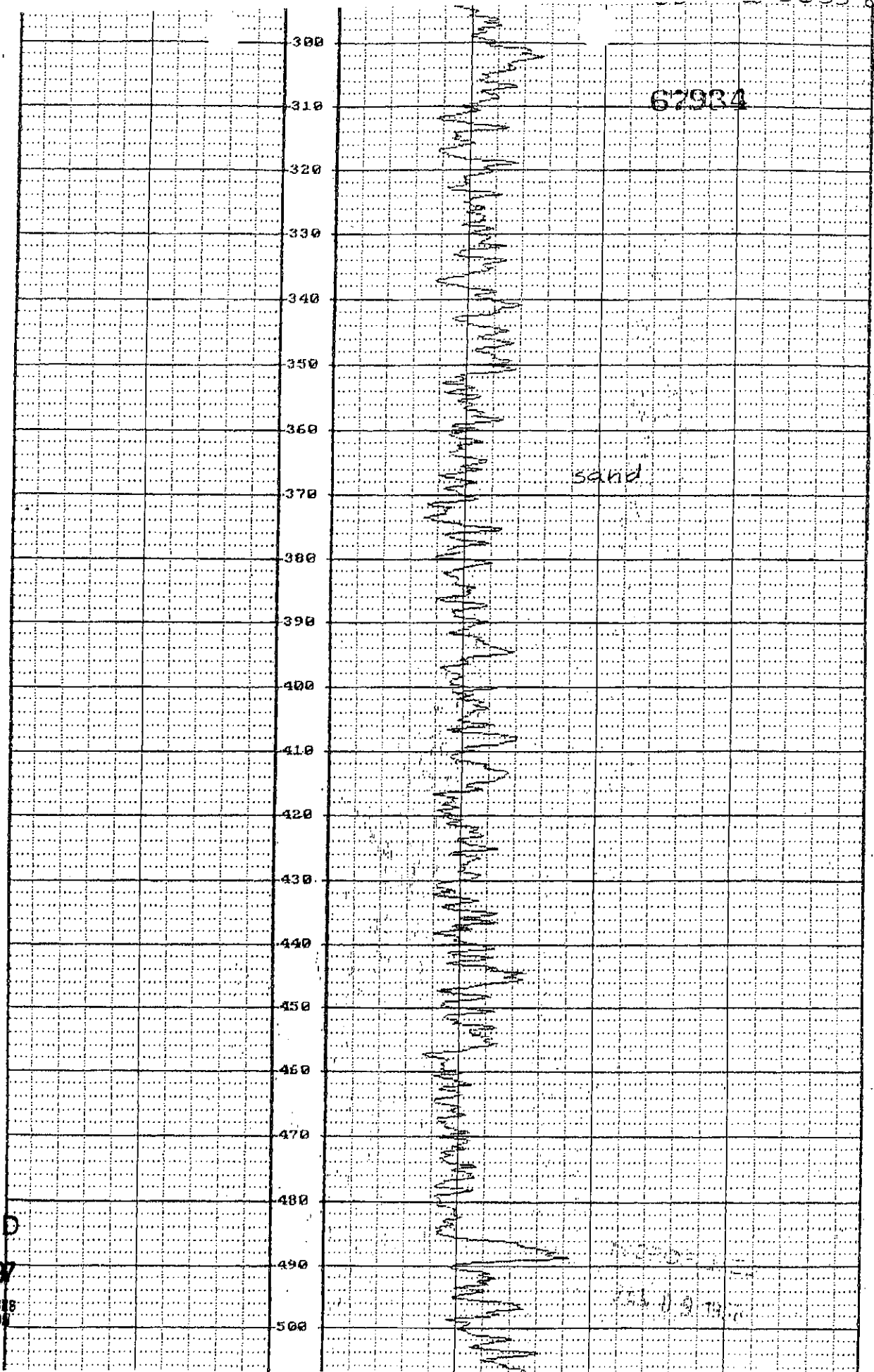
7-2150-100

Aug 3 1954

RECEIVED

OCT 23 1997

WATER RESOURCES  
WESTERN REGION



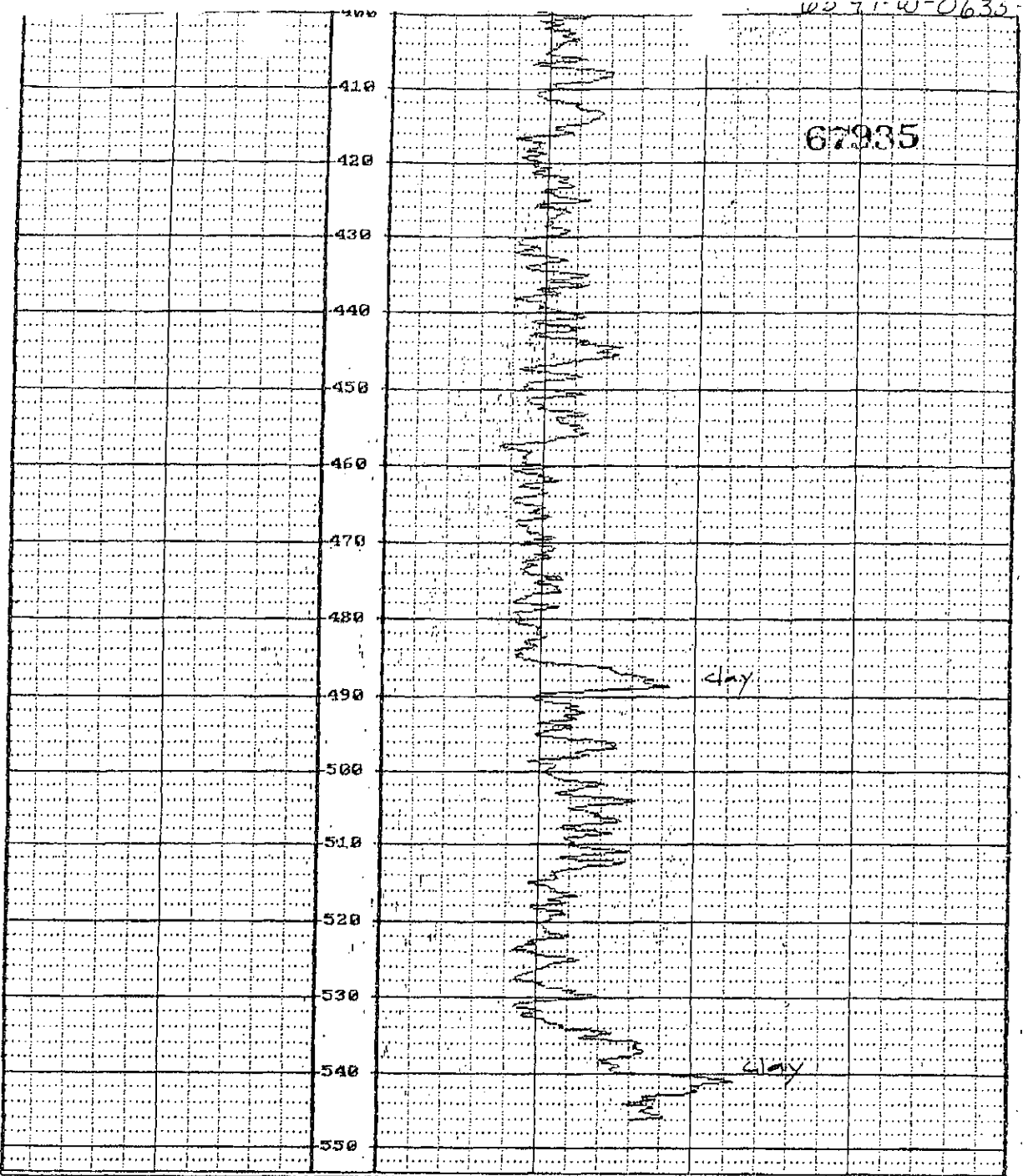
67934

sand

RECEIVED

OCT 23 1997

WATER RESOURCES  
V. CERN REGION



100.04

Gamma 250 CPS

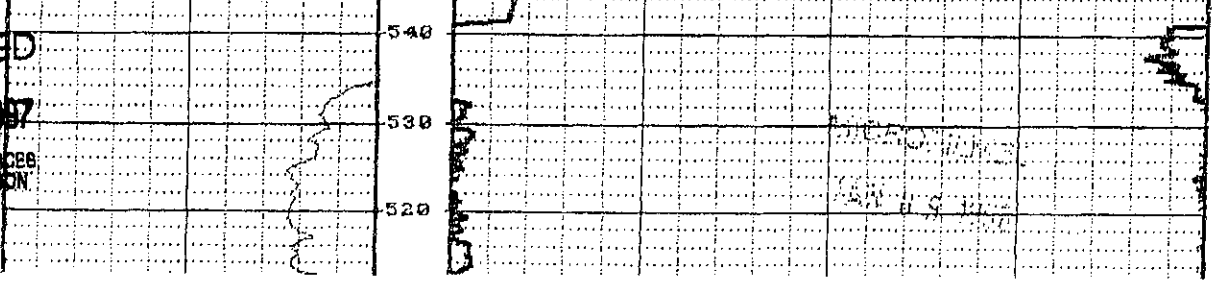
351.25 215.16

SP 100 MU R 200 ohm-m  
SP 100 MU R 200 ohm-m

RECEIVED

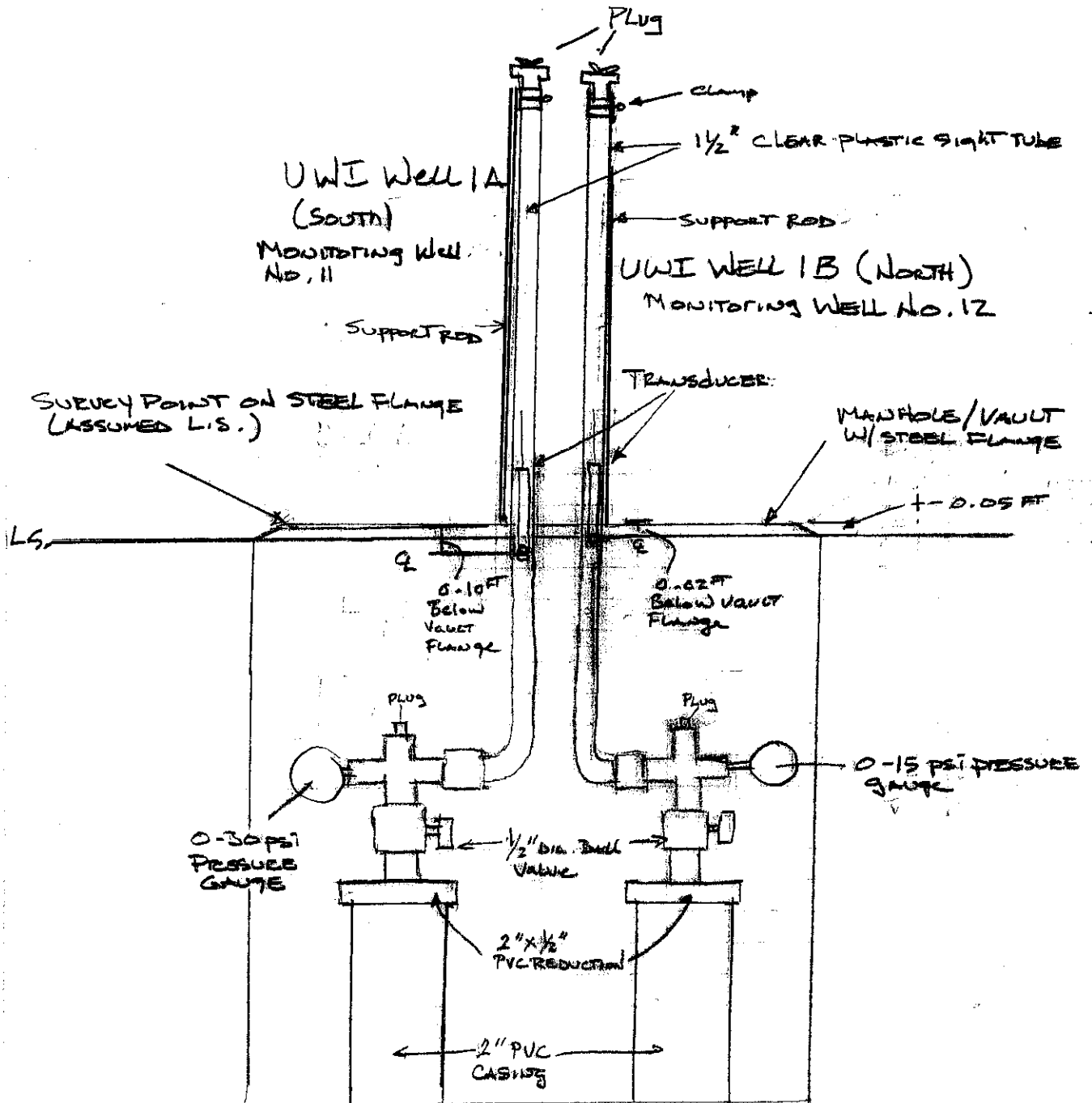
OCT 23 1967

WATER RESOURCES  
WESTERN REGION



# CITY OF EAGLE 1-DAY AQUIFER TEST

MONITORING WELL No. 11 (UWI 1A) AND  
MONITORING WELL No. 12 (UWI 1B)



## **APPENDIX C**

### **MONITORING WELL DATA GRAPHS**

1. Test Well 2 (Eaglefield)(Pumping Well)
2. Test Well 1 (Legacy)
3. Monitoring Well 4 (QCR 4)
4. Monitoring Well 6 (Rick's)
5. Monitoring Well 9 (Strata 1A)
6. Monitoring Well 10 (Strata 1B)
7. Monitoring Well 11 (UWI 1A)
8. Monitoring Well 12 (UWI 1B)
9. Barometer

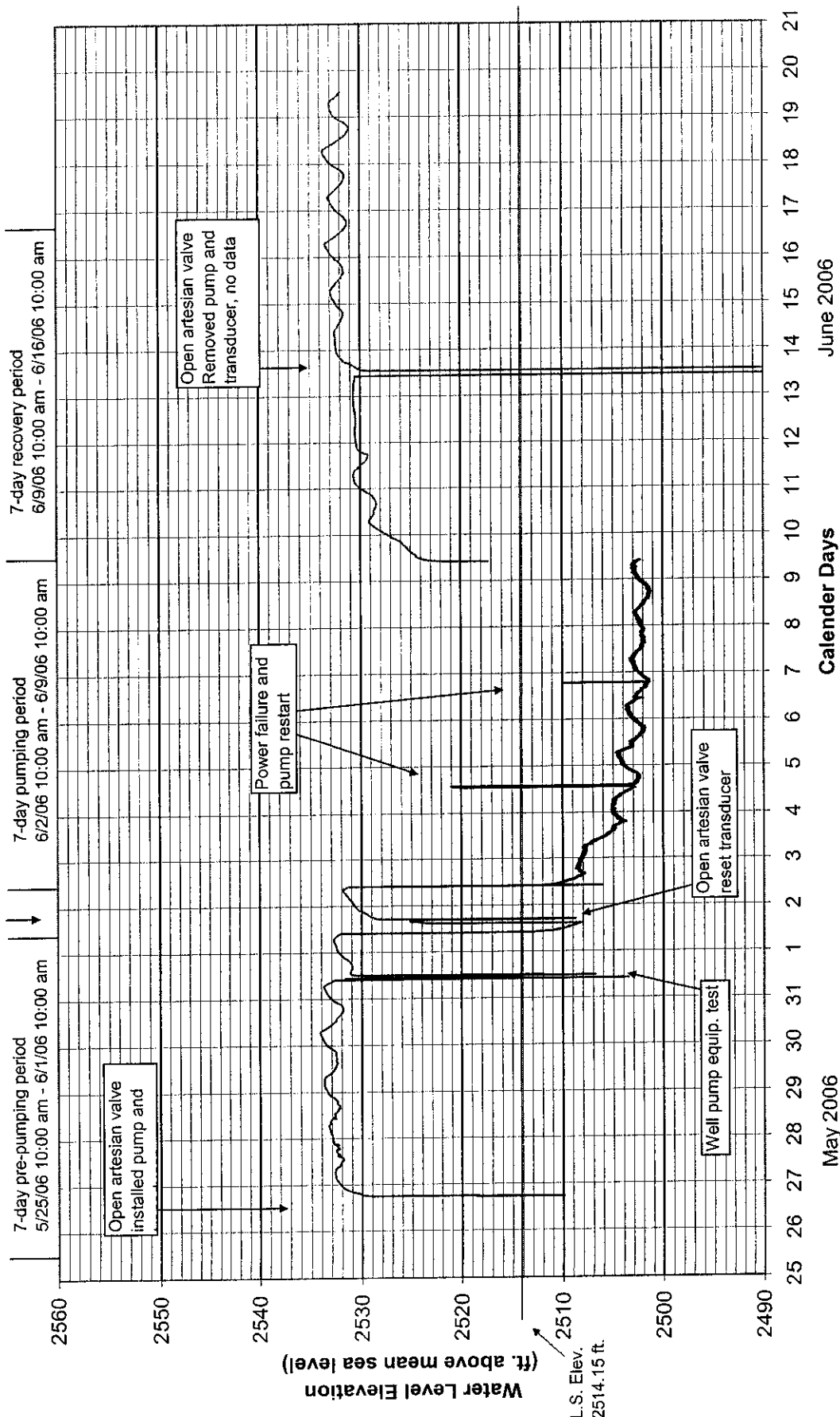
## **Appendix C.1**

**TEST WELL 2 (Eaglefield)(Pumping Well)**

# Pumping Well Test Well No. 2 (Eaglefield Development)

## Water Level Elevation City of Eagle Aquifer Test

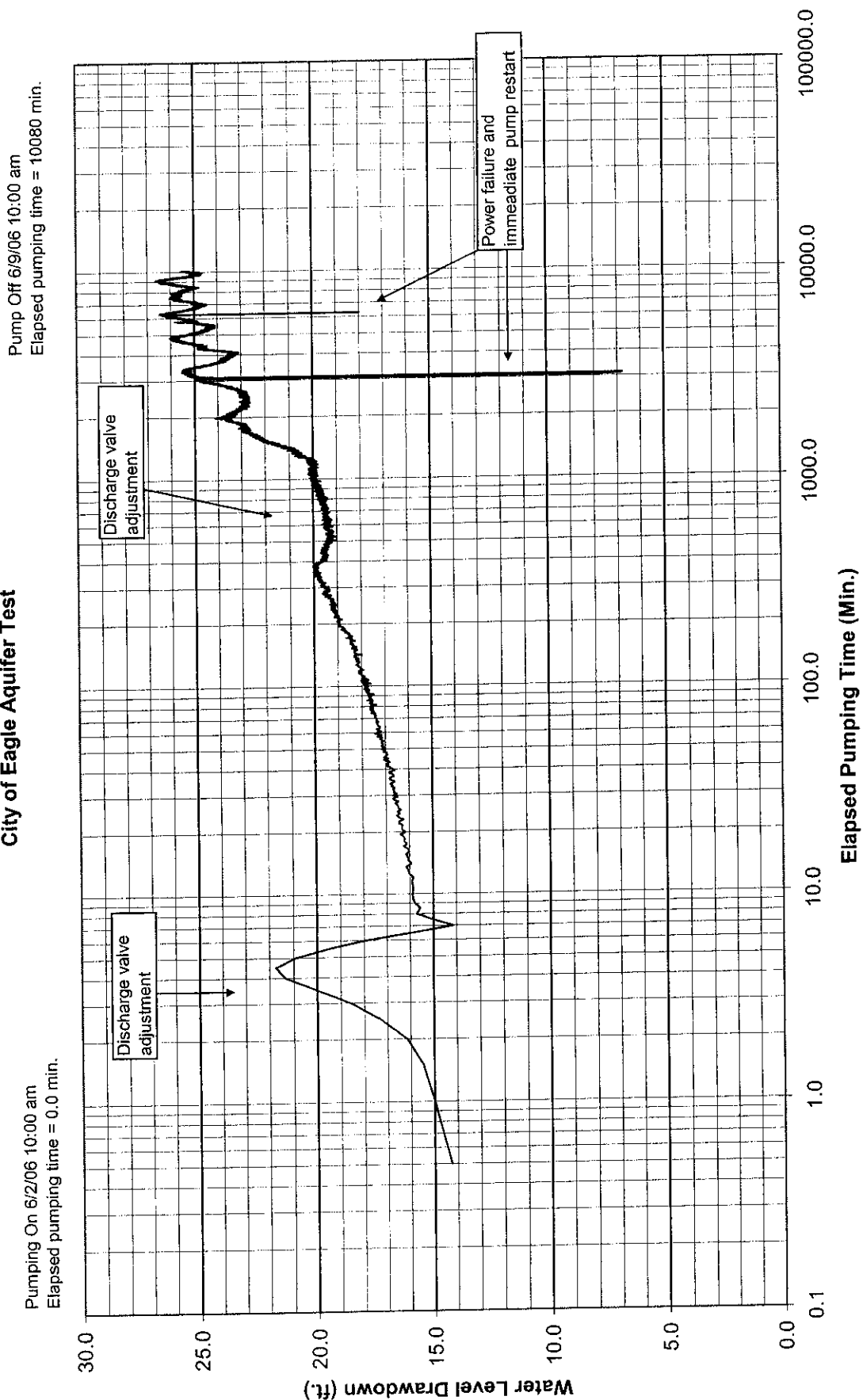
Aborted pumping period  
6/1/06 10:00 am - 6/1/06 3:45 pm



**Pumping Well Test Well No. 2 (Eaglefield Development)**

**Drawdown Semi-log Plot**

**City of Eagle Aquifer Test**



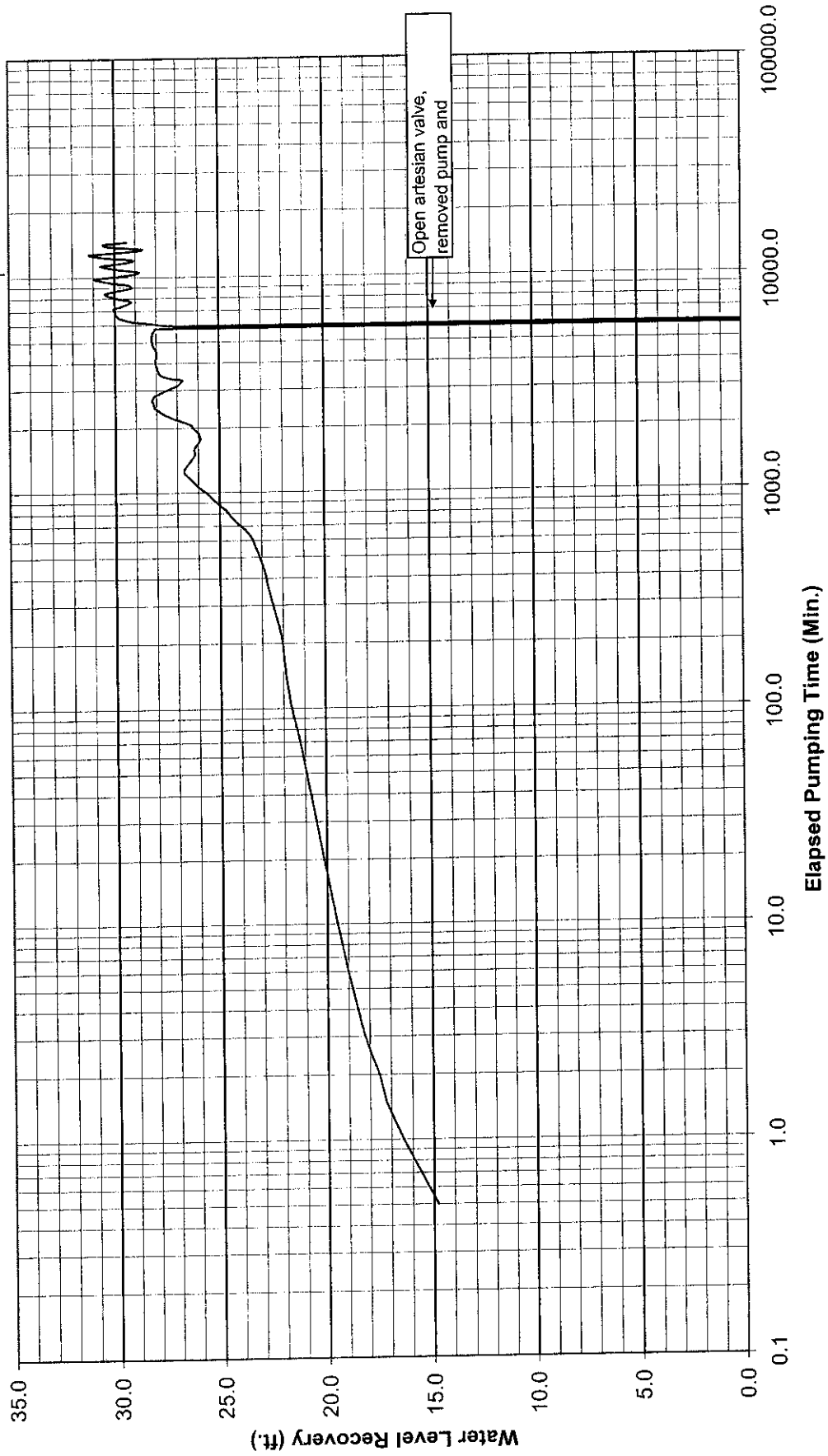
**Pumping Well Test Well No. 2 (Eaglefield Development)**

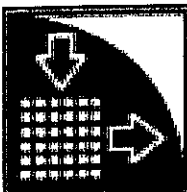
**Recovery Semi-log Plot**

**City of Eagle Aquifer Test**

Pump Off 6/9/06 10:00 am  
Elapsed pumping time = 0 min.

Pumping On 6/16/06 10:00 am  
Elapsed pumping time = 10080 min.





City, State/Province  
Address  
Contact Info  
Company Name

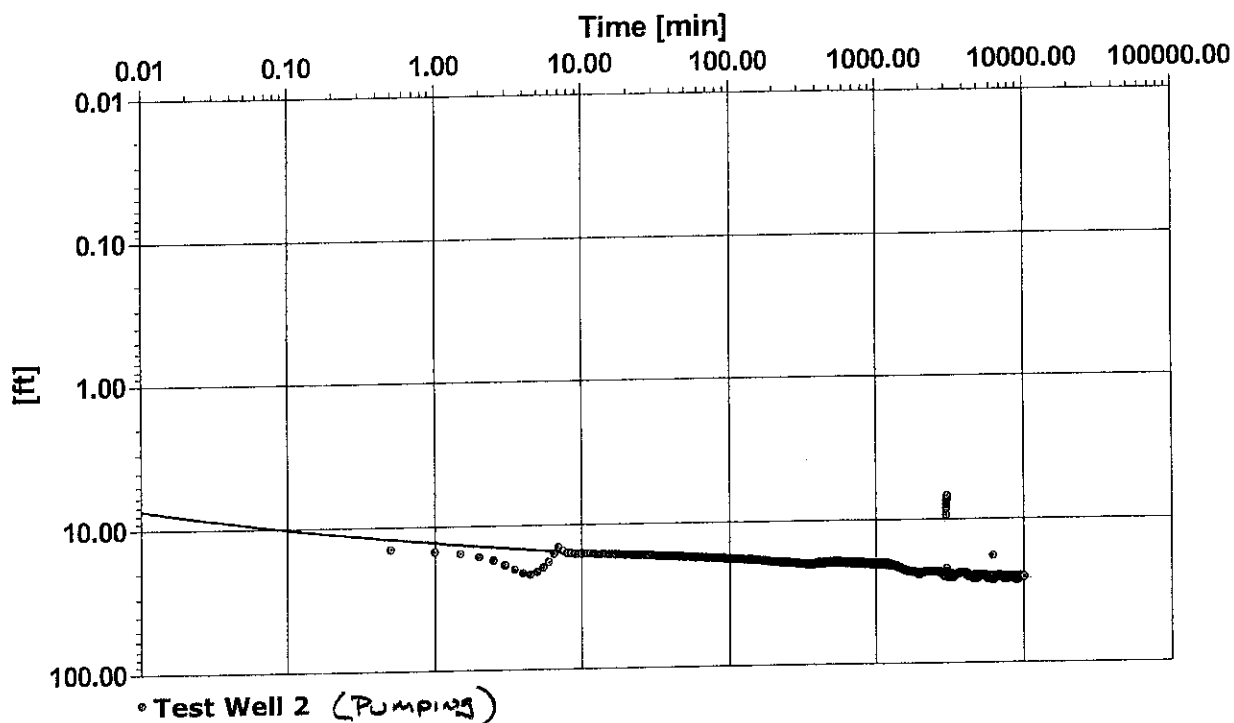
# Pumping Test Analysis Report

Project: Eagle Aquifer Test

Number:

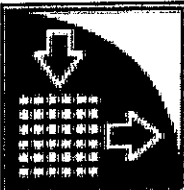
Client:

Location:	Pumping Test: Pumping Test 1	Pumping well: Test Well 2
Test conducted by:		Test date: 6/29/2006
Analysis performed by:	New analysis 1	Date: 6/29/2006
Aquifer Thickness: 100.00 ft	Discharge rate: 1580 [U.S. gal/min]	



## Calculation after Theis

Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]	
Test Well 2	$2.00 \times 10^4$	$2.00 \times 10^2$	$2.93 \times 10^{-3}$	0.5	



City, State/Province  
Address  
Contact Info  
Company Name

# Pumping Test Analysis Report

Project: Recovery Eagle Aquifer Test

Number:

Client:

Location:

Pumping Test: Recovery Eagle Aquifer Test

Pumping well: TestwellNo2 R

Test conducted by:

Test date: 6/30/2006

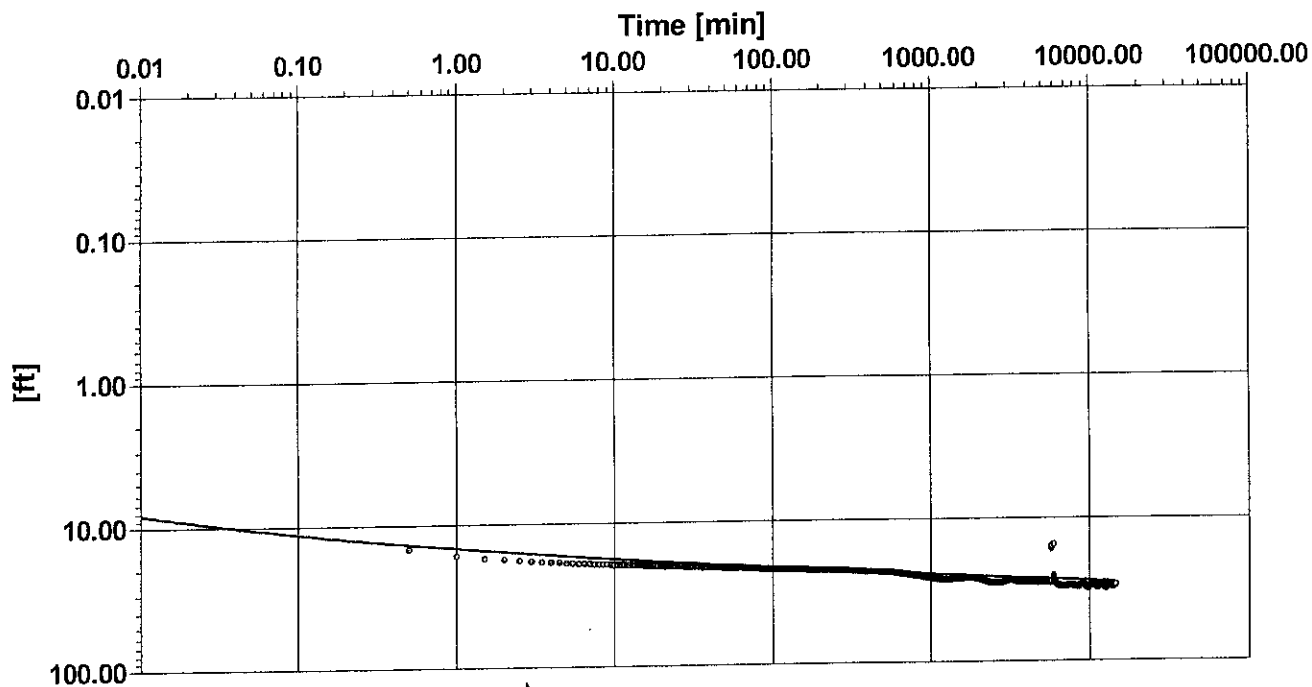
Analysis performed by:

recovery

Date: 6/30/2006

Aquifer Thickness: 100.00 ft

Discharge rate: 1580 [U.S. gal/min]



## Calculation after Theis

Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]	
TestwellNo2 R	$1.75 \times 10^4$	$1.75 \times 10^2$	$3.00 \times 10^{-3}$	0.5	

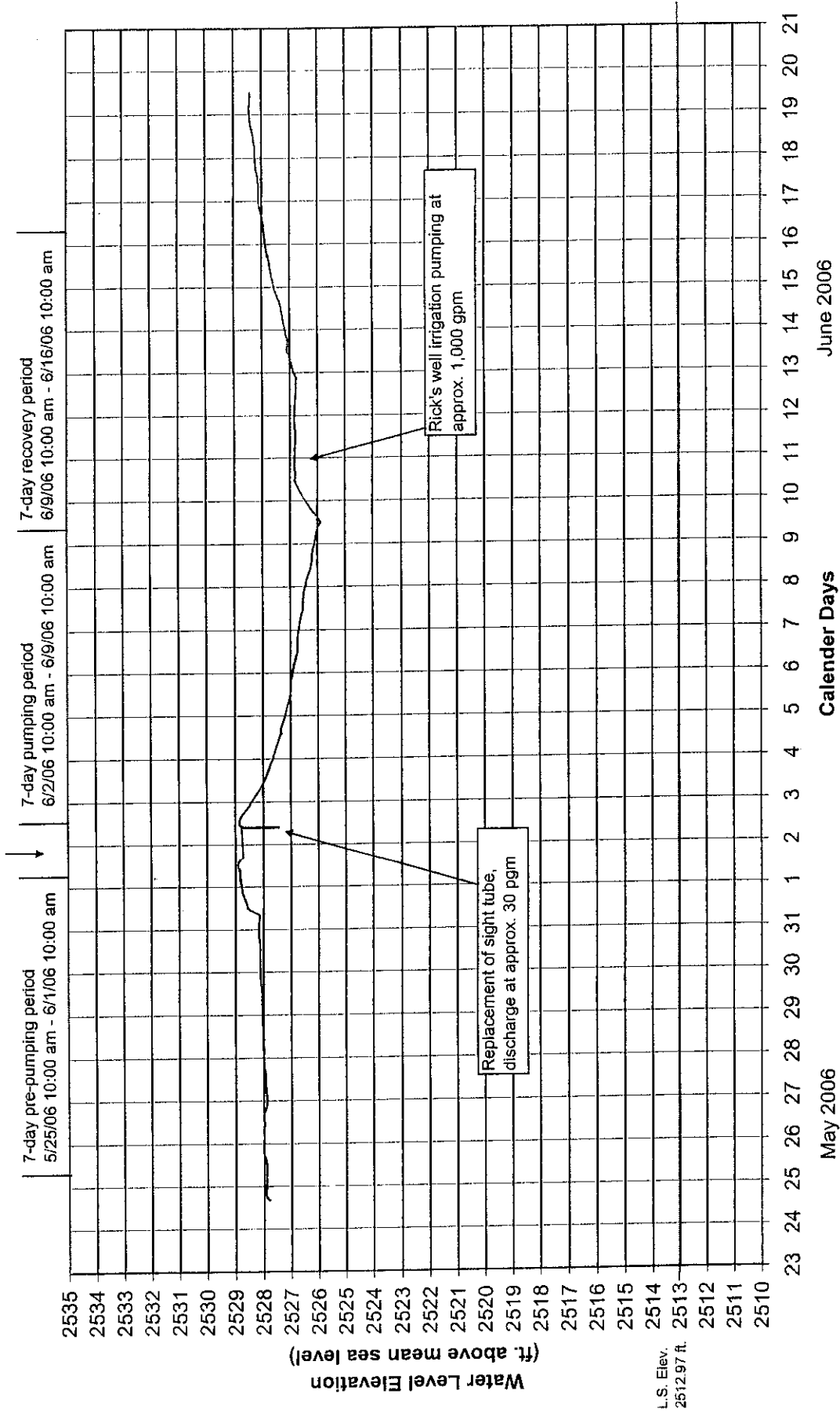
## Appendix C.2

### TEST WELL 1 (Legacy)

# Monitoring Well Test Well No. 1 (Legacy Development)

## Water Level Elevation City of Eagle Aquifer Test

Aborted pumping period  
6/1/06 10:00 am - 6/1/06 3:45 pm



L.S. Elev.  
2512.97 ft.

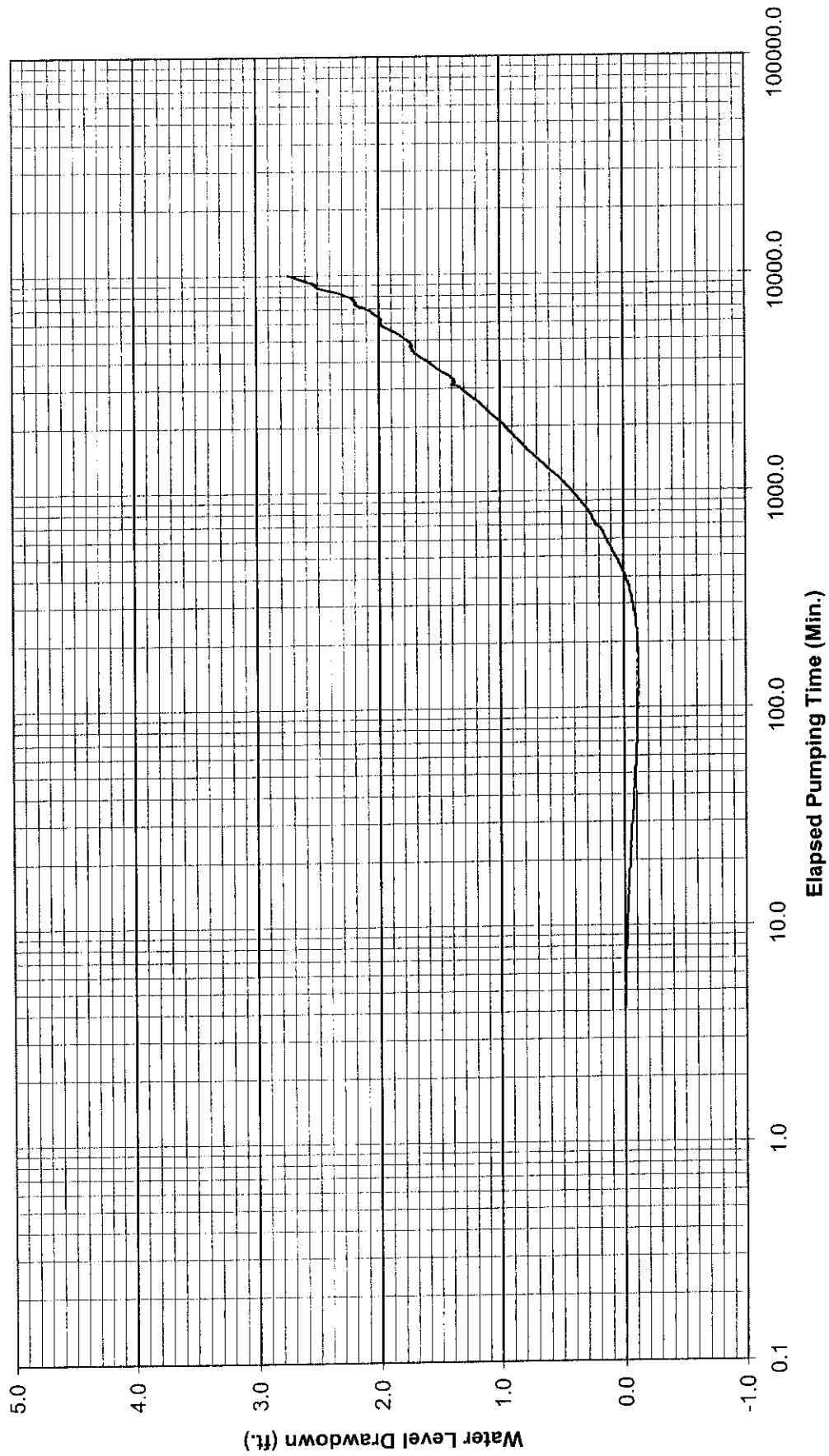
# Monitoring Well Test Well No. 1 (Legacy Development)

## Drawdown Semi-log Plot

### City of Eagle Aquifer Test

Pump Off 6/9/06 10:00 am  
Elapsed pumping time = 10080 min.

Pumping On 6/2/06 10:00 am  
Elapsed pumping time = 0.0 min.



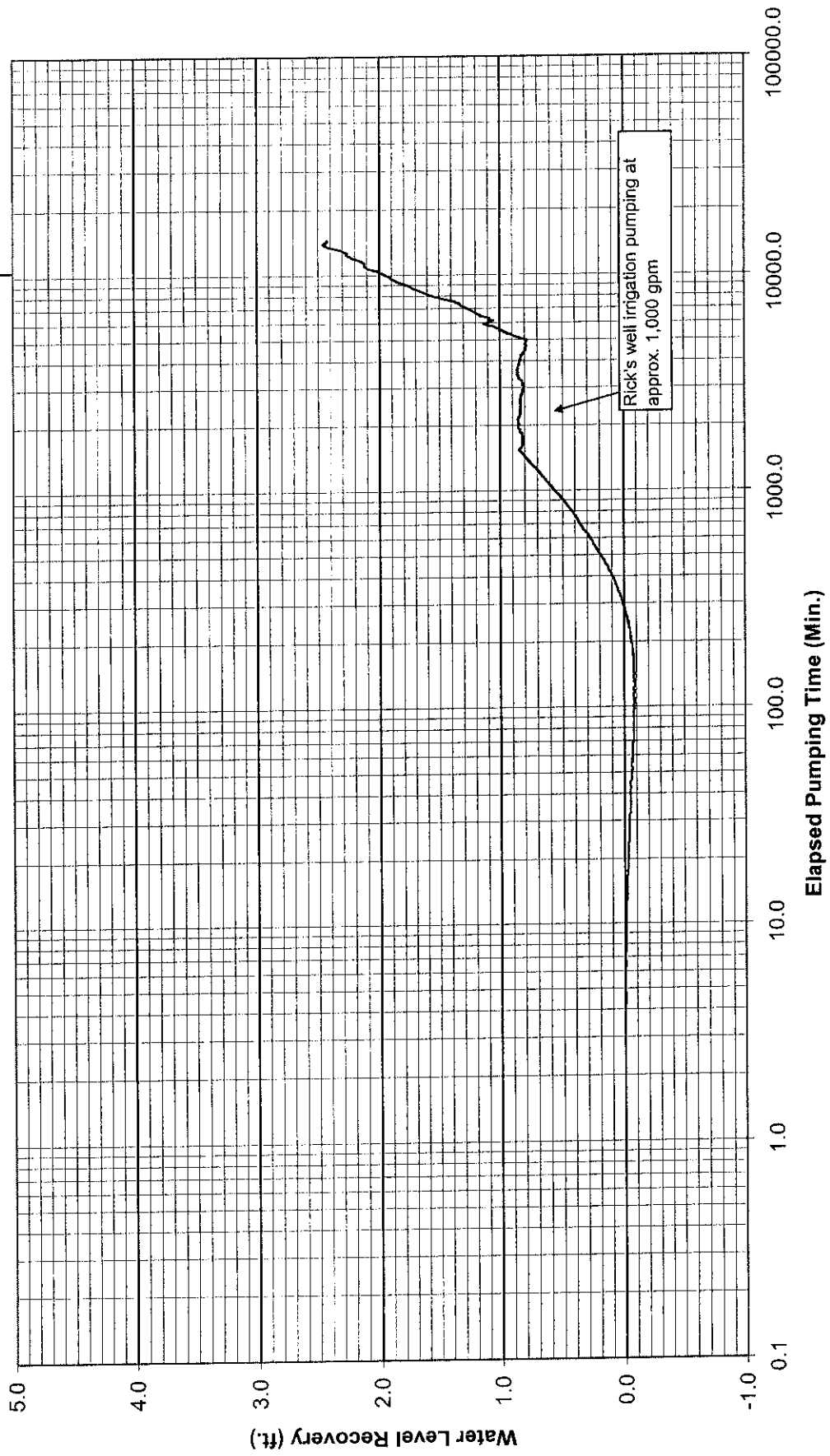
# Monitoring Well Test Well No. 1 (Legacy Development)

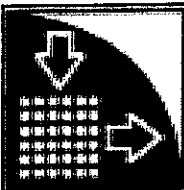
## Recovery Semi-log Plot

### City of Eagle Aquifer Test

Pumping On 6/9/06 10:00 am  
Elapsed pumping time = 0.0 min.

Pump Off 6/16/06 10:00 am  
Elapsed pumping time = 10080 min.





City, State/Province  
Address  
Contact Info  
Company Name

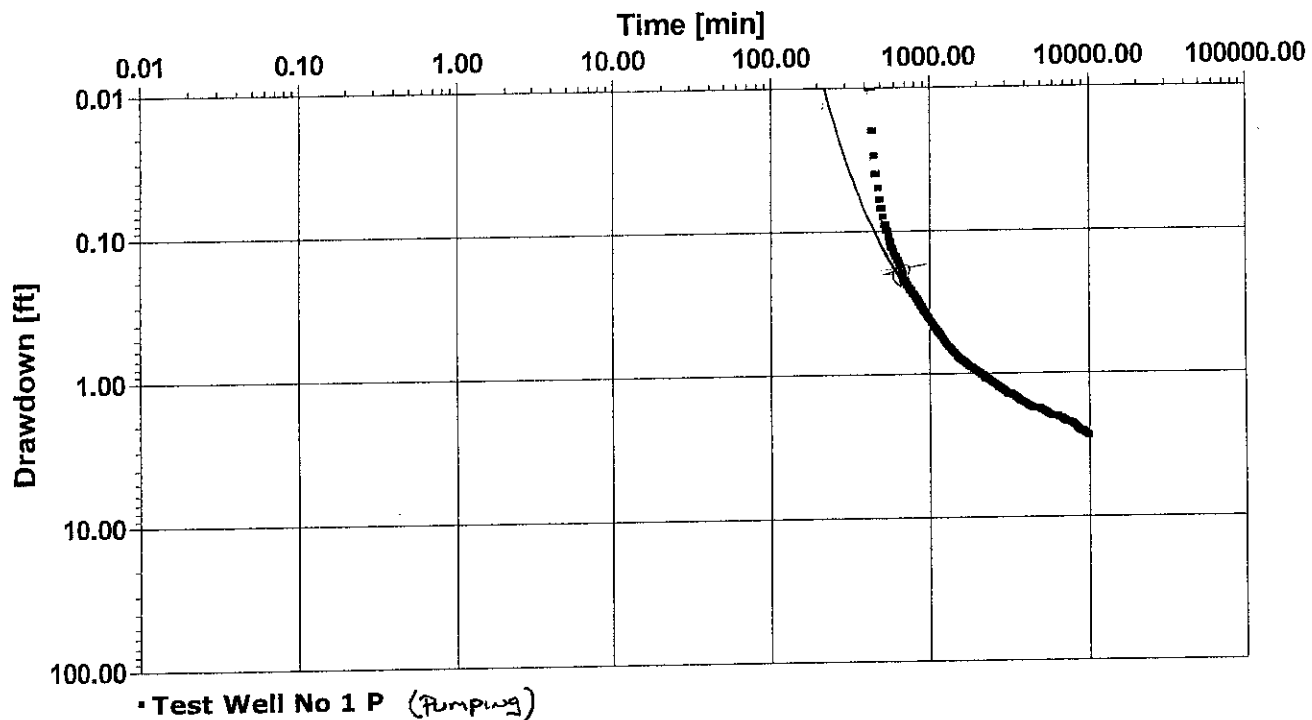
# Pumping Test Analysis Report

Project: Eagle Aquifer Test

Number:

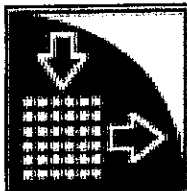
Client:

Location:	Pumping Test: Drawdown	Pumping well: Test Well No 2 P
Test conducted by:		Test date: 6/30/2006
Analysis performed by:	New analysis 1	Date: 6/30/2006
Aquifer Thickness: 100.00 ft	Discharge rate: 1580 [U.S. gal/min]	



## Calculation after Theis

Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]	
Test Well No 1 P	$1.95 \times 10^4$	$1.95 \times 10^2$	$1.58 \times 10^{-2}$	1604.58	



City, State/Province  
Address  
Contact Info  
Company Name

# Pumping Test Analysis Report

Project: Recovery Eagle Aquifer Test

Number:

Client:

Location:

Pumping Test: Recovery Eagle Aquifer Test

Pumping well: TestwellNo2 R

Test conducted by:

Test date: 6/30/2006

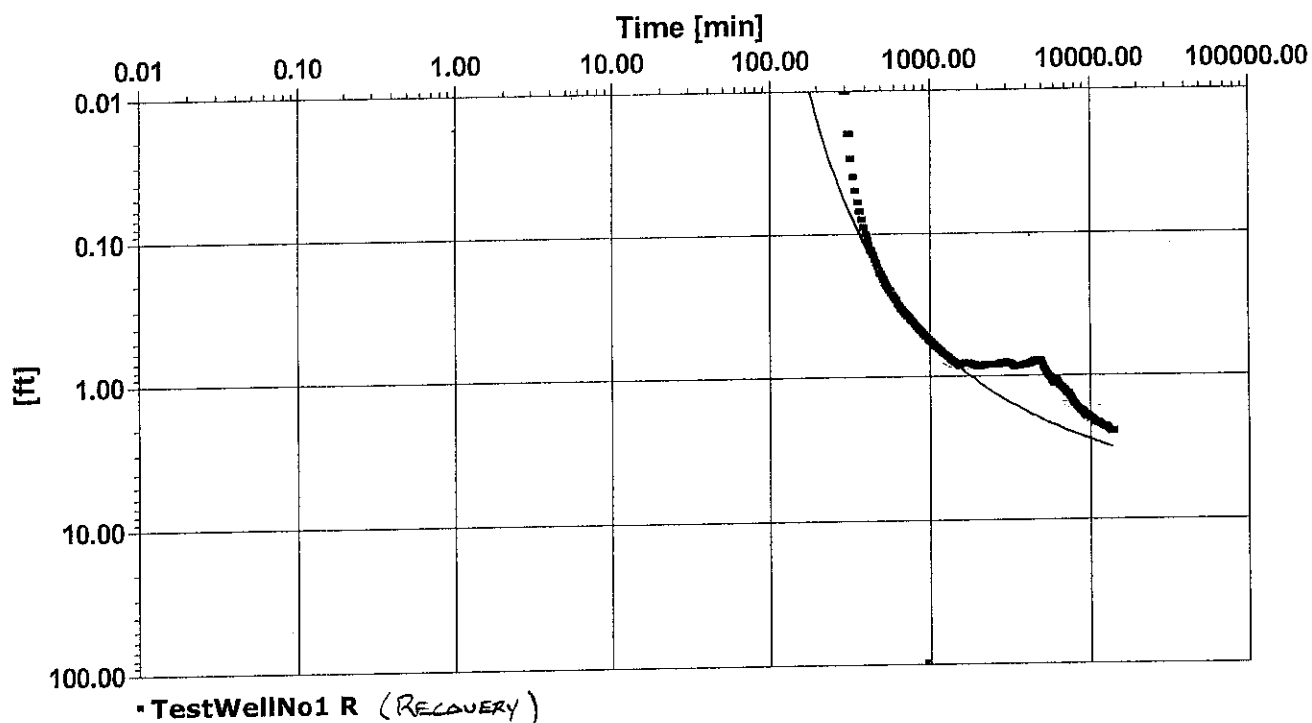
Analysis performed by:

recovery

Date: 6/30/2006

Aquifer Thickness: 100.00 ft

Discharge rate: 1580 [U.S. gal/min]



## Calculation after Theis

Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]	
TestWellNo1 R	$2.01 \times 10^4$	$2.01 \times 10^2$	$1.30 \times 10^{-2}$	1604.58	

## Appendix C.3

### MONITORING WELL 4 (QCR 4)

Water Level Elevation (ft. above mean sea level)

Calendar Days

7-day pre-pumping period  
5/25/06 10:00 am - 6/1/06 10:00 am

7-day pumping period  
6/2/06 10:00 am - 6/9/06 10:00 am

7-day recovery period  
6/9/06 10:00 am - 6/16/06 10:00 am

L.S. Elev.  
2501.86 ft.

May 2006

June 2006

## **Appendix C.4**

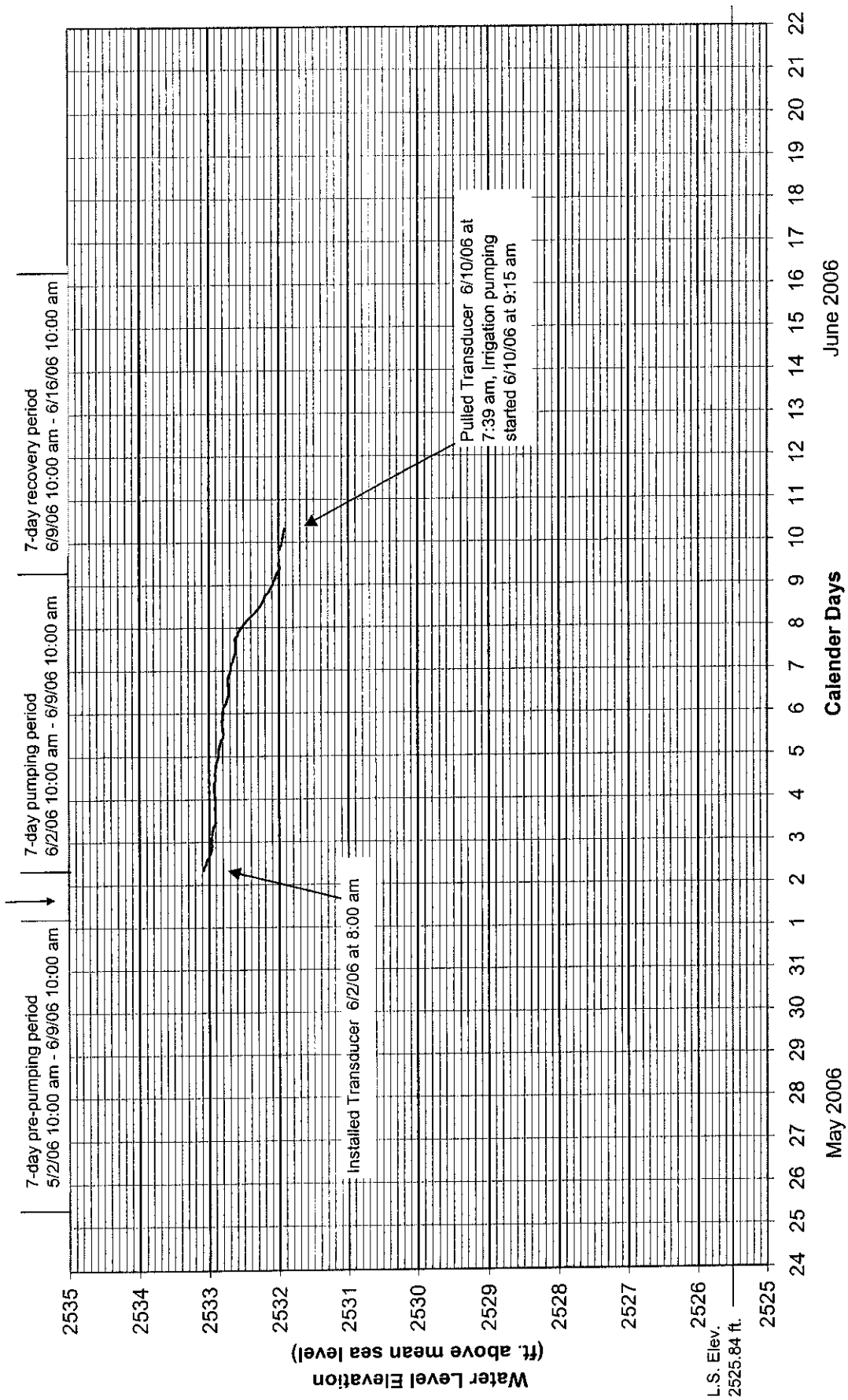
### **MONITORING WELL 6 (Rick's)**

**Monitoring Well No. 6 (Rick's Well)**

**Water Level Elevation**

**City of Eagle Aquifer Test**

Aborted pumping period  
6/1/06 10:00 am - 6/1/06 3:45 pm



## Appendix C.5

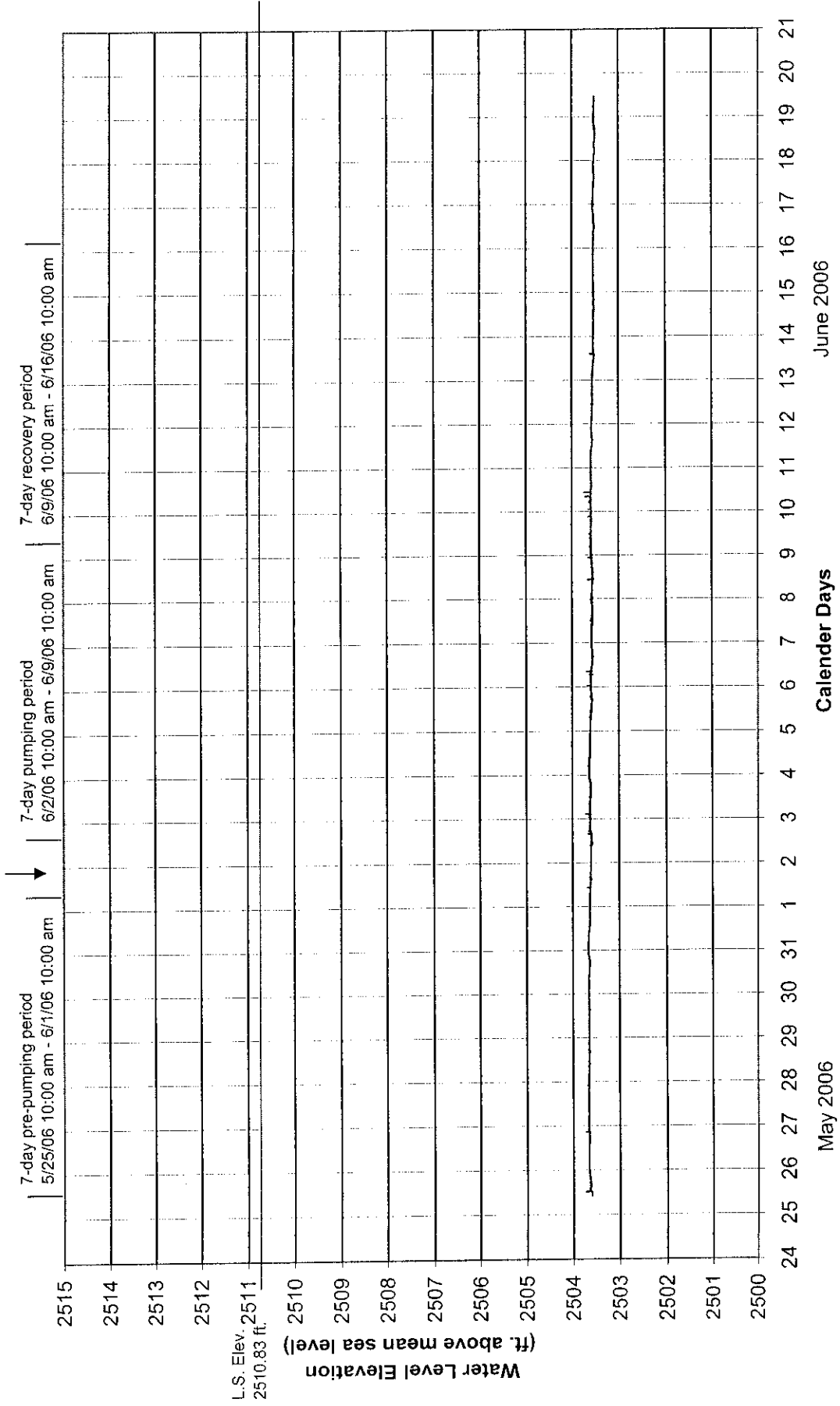
### MONITORING WELL 9 (Strata 1A)

**Monitoring Well No. 9 (Strata Well 1A)**

**Water Level Elevation**

**City of Eagle Aquifer Test**

Aborted pumping period  
6/1/06 10:00 am - 6/1/06 3:45 pm



L.S. Elev. 2511  
2510.83 ft.

## Appendix C.6

### MONITORING WELL 10 (Strata Well 1B)

**Monitoring Well No. 10 (Strata Well 1B)**

**Water Level Elevation**

**City of Eagle Aquifer Test**

Aborted pumping period  
6/1/06 10:00 am - 6/1/06 3:45 pm

7-day pre-pumping period  
5/25/06 10:00 am - 6/1/06 10:00 am

7-day pumping period  
6/2/06 10:00 am - 6/9/06 10:00 am

7-day recovery period  
6/9/06 10:00 am - 6/16/06 10:00 am

2515

2514

2513

2512

L.S. Elev.  
2510.52 ft

2510

2509

2508

2507

2506

2505

2504

2503

2502

2501

2500

Water Level Elevation  
(ft. above mean sea level)

24

25

26

27

28

29

30

31

1

2

3

4

5

6

7

8

9

10

11

12

13

14

15

16

17

18

19

20

21

May 2006

Calendar Days

June 2006

## **Appendix C.7**

### **MONITORING WELL 11 (UWI 1A)**

**Monitoring Well No. 11 (UWI 1A South)**

**Water Level Elevation**

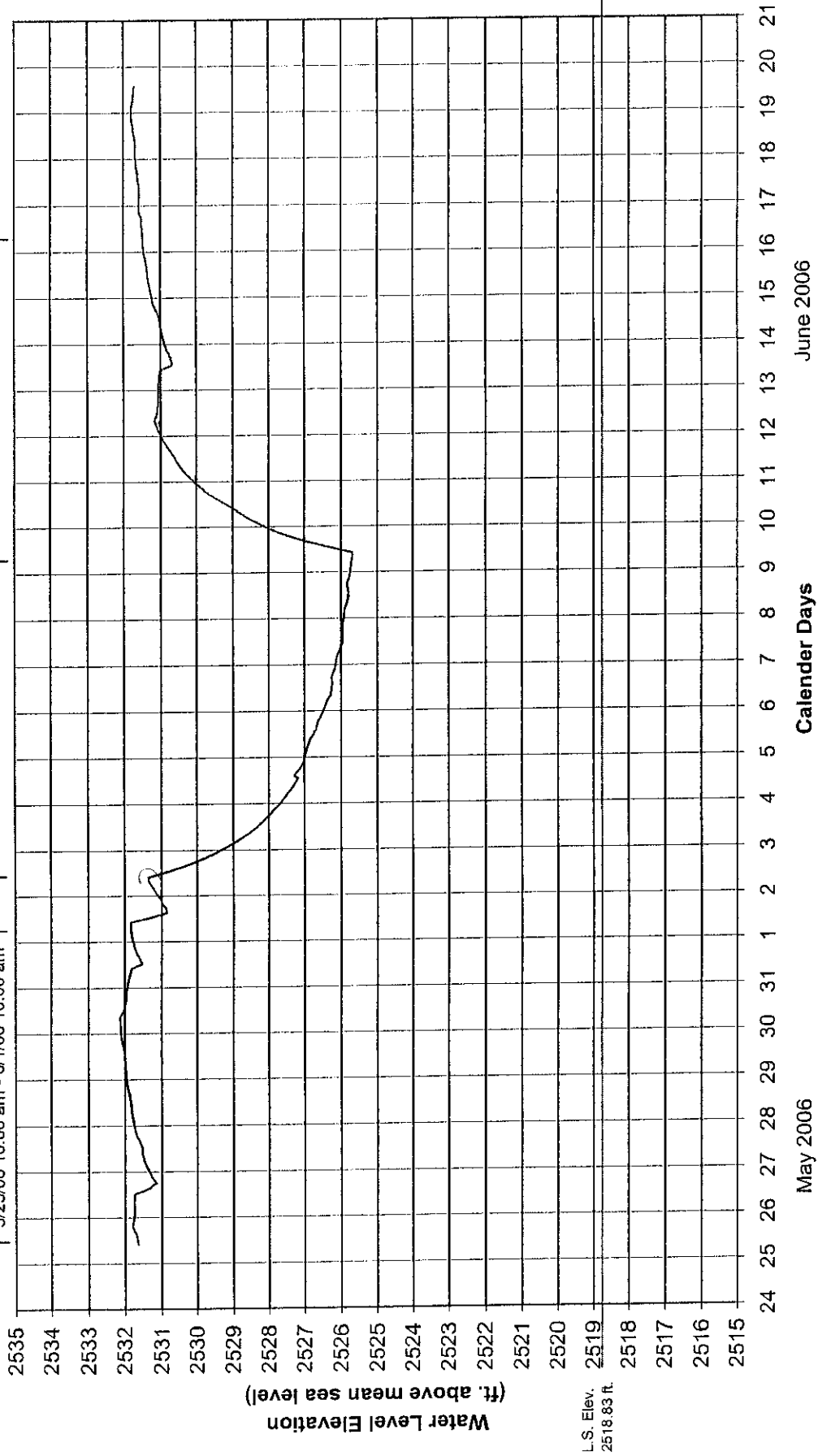
**City of Eagle Aquifer Test**

Aborted pumping period  
6/1/06 10:00 am - 6/1/06 3:45 pm

7-day pre-pumping period  
5/25/06 10:00 am - 6/1/06 10:00 am

7-day pumping period  
6/2/06 10:00 am - 6/9/06 10:00 am

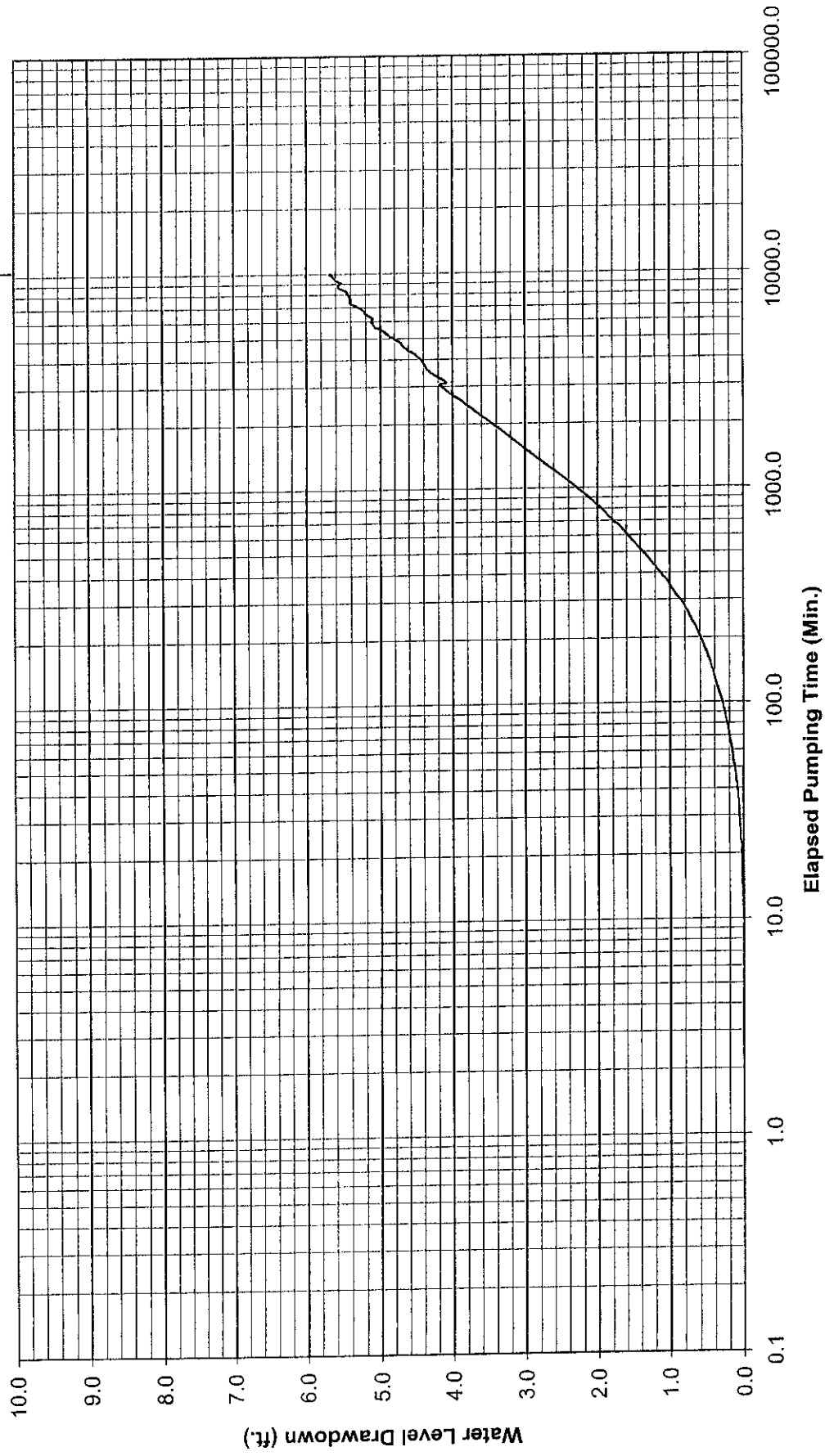
7-day recovery period  
6/9/06 10:00 am - 6/16/06 10:00 am



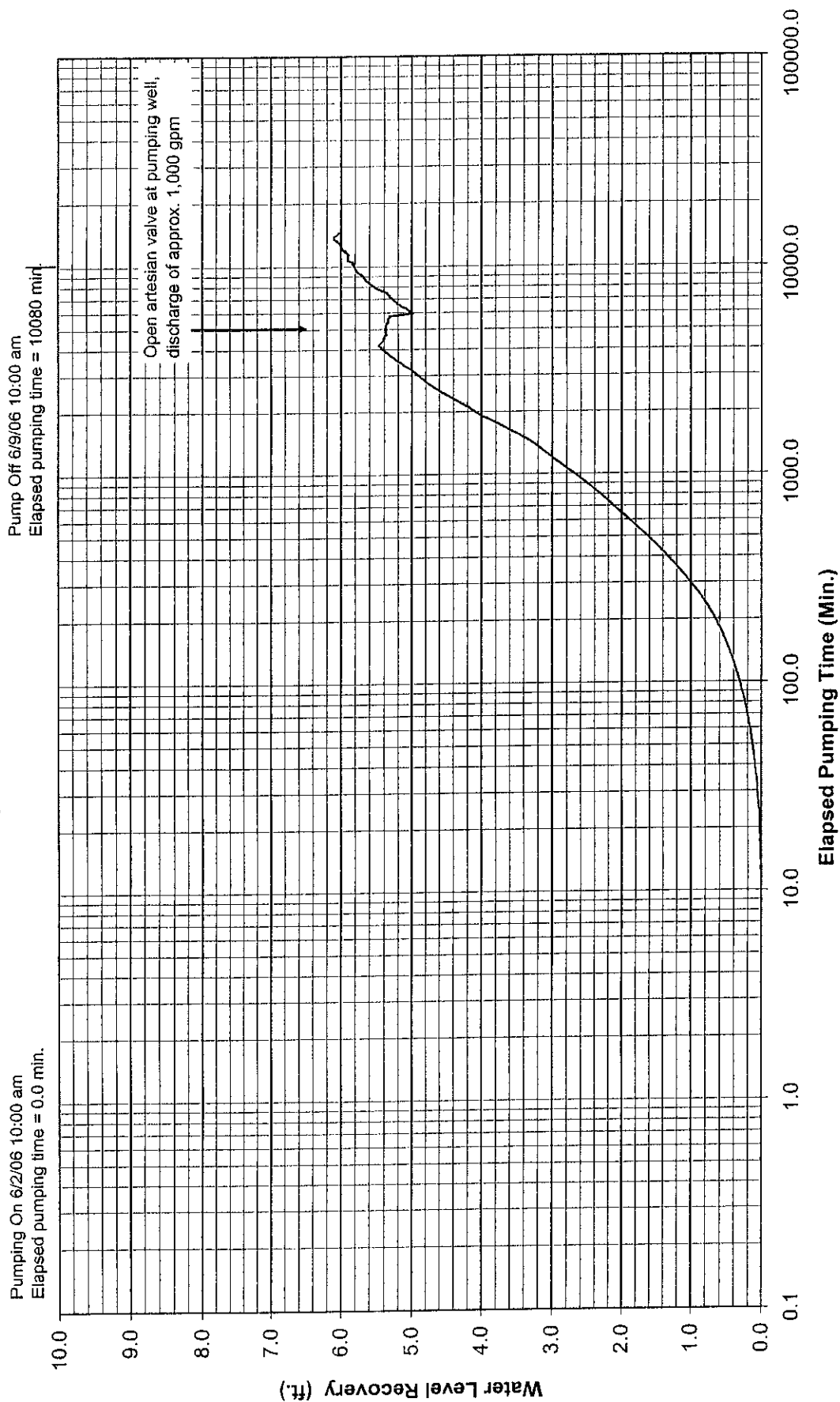
**Monitoring Well No. 11 (UWI 1A South)**  
**Drawdown Semi-log Plot**  
**City of Eagle Aquifer Test**

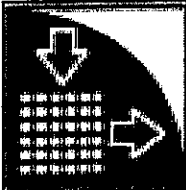
Pumping On 6/2/06 10:00 am  
Elapsed pumping time = 0.0 min.

Pump Off 6/9/06 10:00 am  
Elapsed pumping time = 10080 min.



Monitoring Well No. 11 (UW1 1A South)  
Recovery Semi-log Plot  
City of Eagle Aquifer Test





City, State/Province  
Address  
Contact Info  
Company Name

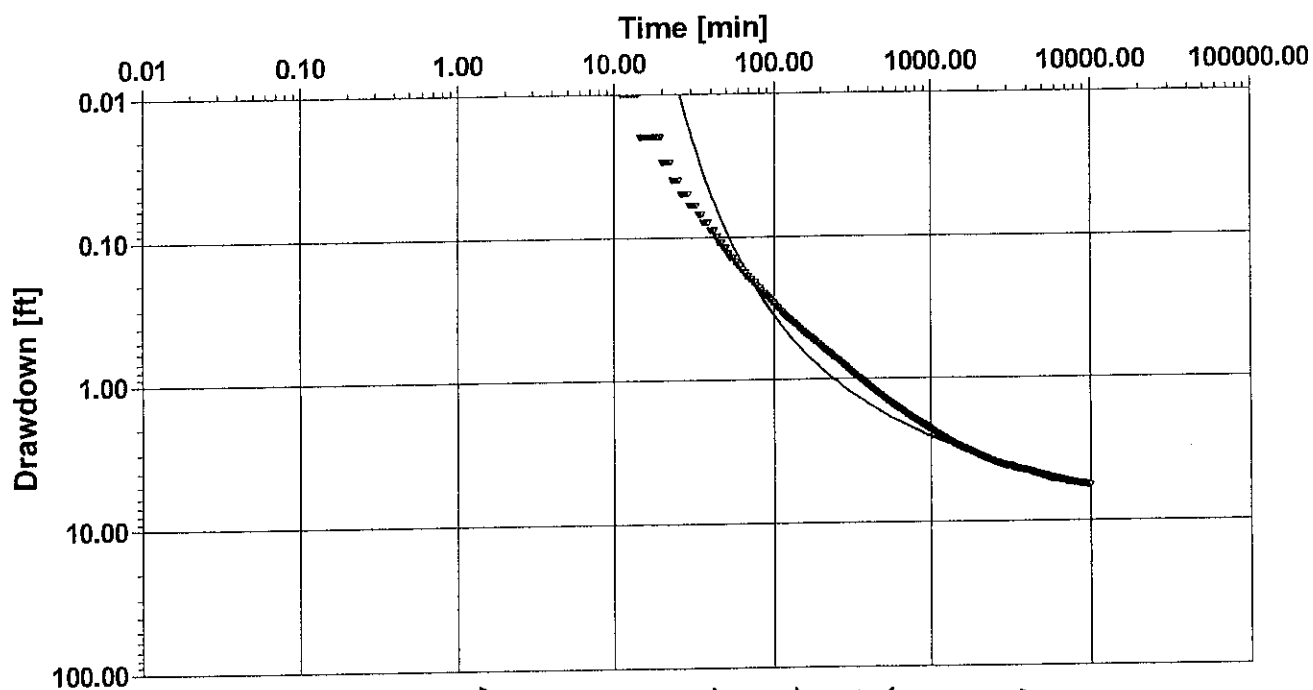
# Pumping Test Analysis Report

Project: Eagle Aquifer Test

Number:

Client:

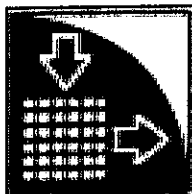
Location:	Pumping Test: Drawdown	Pumping well: Test Well No 2 P
Test conducted by:		Test date: 6/30/2006
Analysis performed by:	New analysis 1	Date: 6/30/2006
Aquifer Thickness: 100.00 ft	Discharge rate: 1580 [U.S. gal/min]	



• UWI South P (Pumping) Monitoring Well No. 11 (UWI 1A)

## Calculation after Theis

Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]	
UWI South P	$1.85 \times 10^4$	$1.85 \times 10^2$	$7.80 \times 10^{-4}$	2405.68	



City, State/Province  
Address  
Contact Info  
Company Name

# Pumping Test Analysis Report

Project: Recovery Eagle Aquifer Test

Number:

Client:

Location:

Pumping Test: Recovery Eagle Aquifer Test

Pumping well: TestwellNo2 R

Test conducted by:

Test date: 6/30/2006

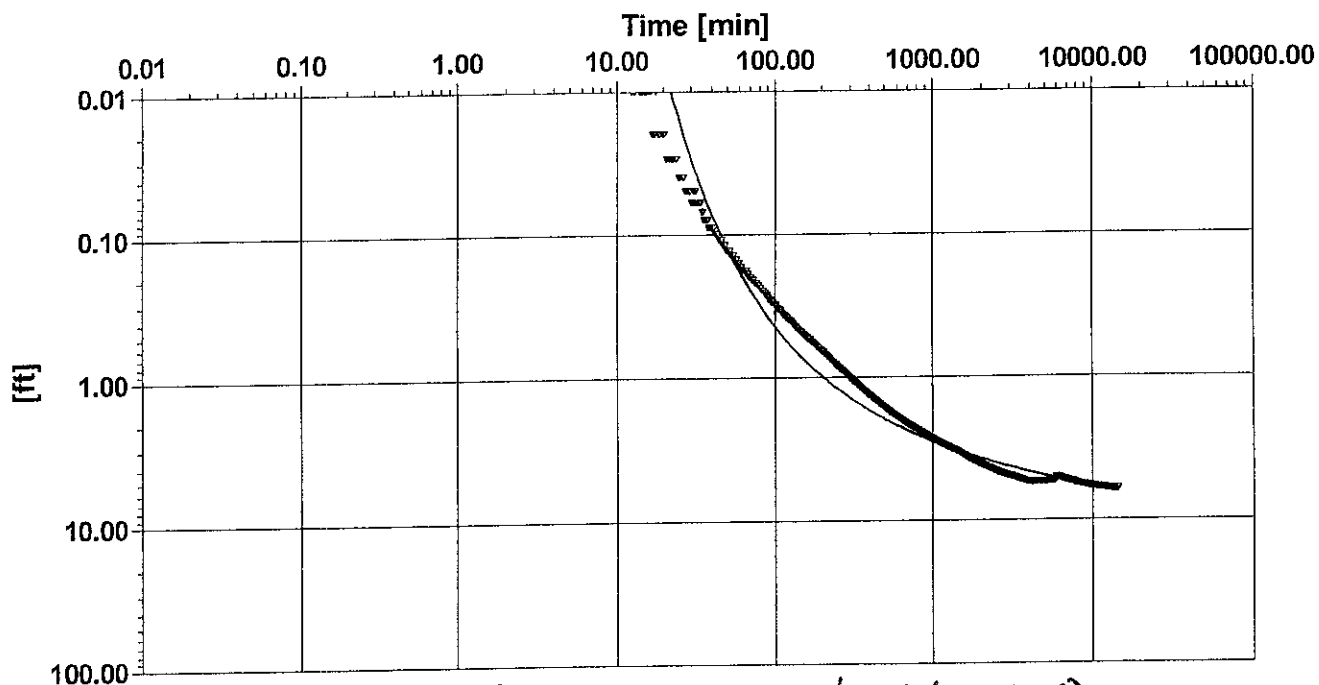
Analysis performed by:

recovery

Date: 6/30/2006

Aquifer Thickness: 100.00 ft

Discharge rate: 1580 [U.S. gal/min]



• UWISouth R (RECOVERY) MONITORING WELL No. 11 (UWI 1B)

Calculation after Theis

Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]	
UWISouth R	$1.77 \times 10^4$	$1.77 \times 10^2$	$6.62 \times 10^{-4}$	2405.68	

Exclusion of Points  
 $t = 2000$  to  $t = 4257$

COMPUTER FIT

## Appendix C.8

### MONITORING WELL 12 (UWI 1B)

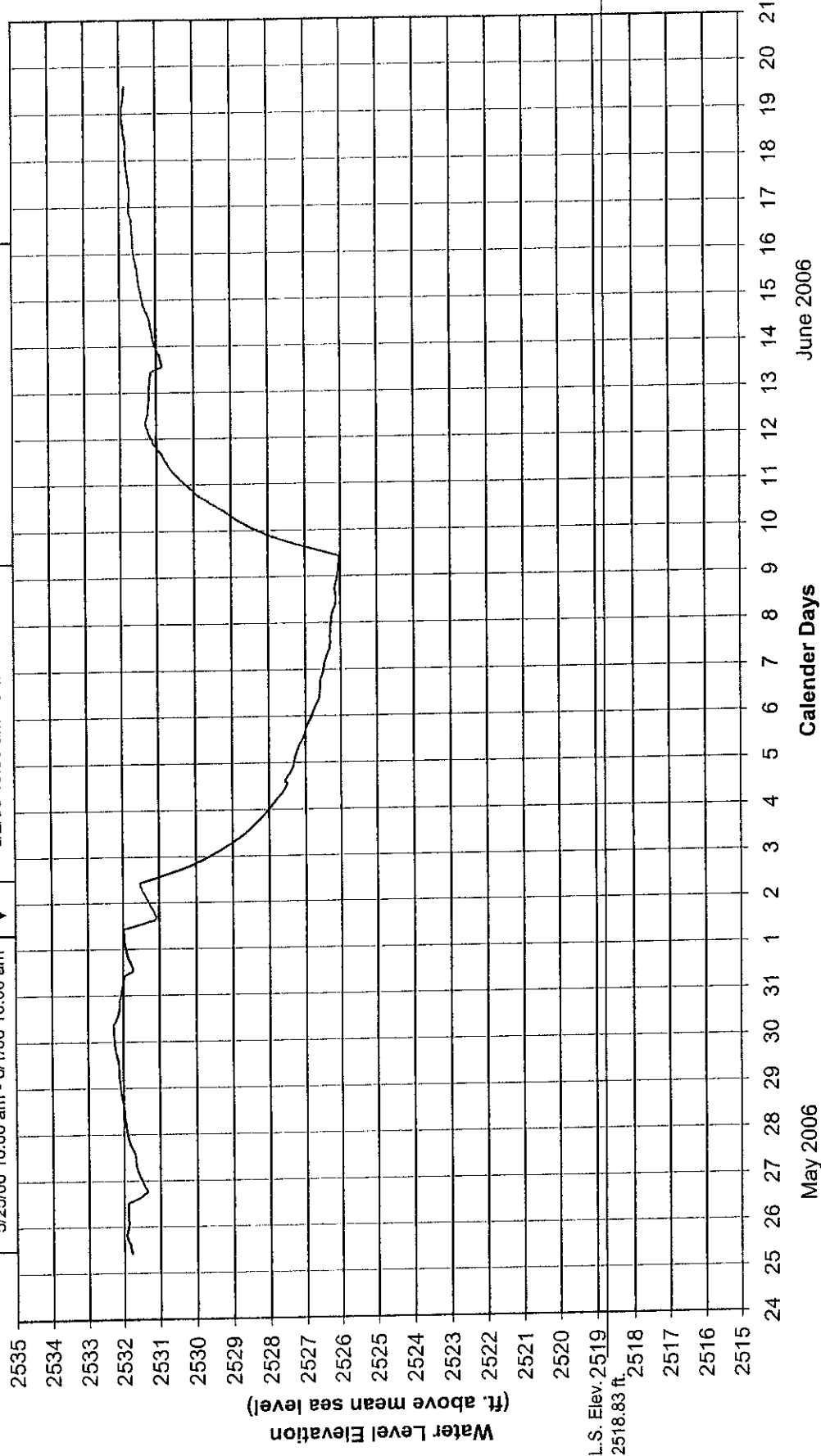
**Monitoring Well No. 12 (UWI 1B North)**  
**Water Level Elevation**  
**City of Eagle Aquifer Test**

Aborted pumping period  
 6/1/06 10:00 am - 6/1/06 3:45 pm

7-day pre-pumping period  
 5/25/06 10:00 am - 6/1/06 10:00 am

7-day pumping period  
 6/2/06 10:00 am - 6/9/06 10:00 am

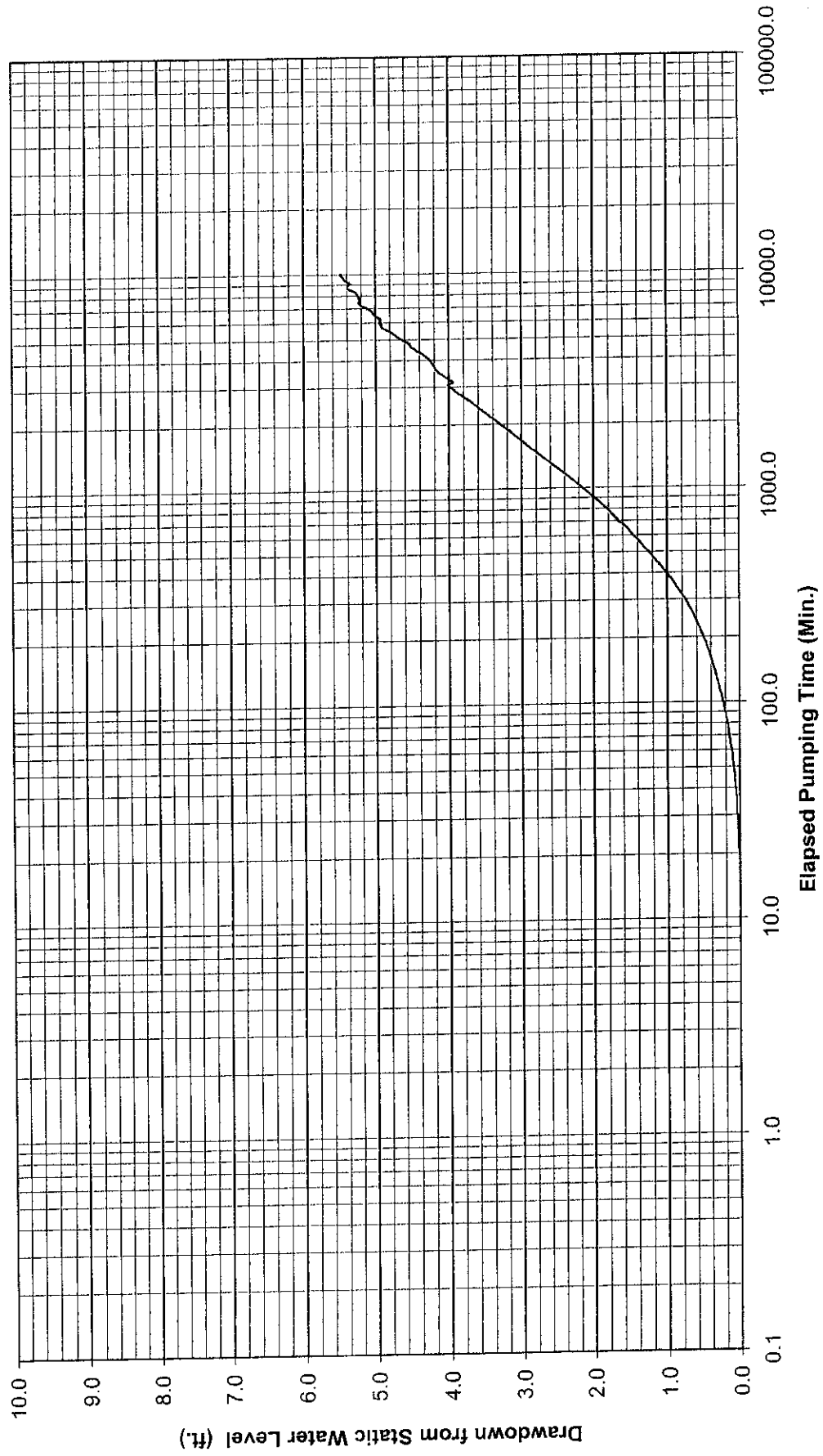
7-day recovery period  
 6/9/06 10:00 am - 6/16/06 10:00 am



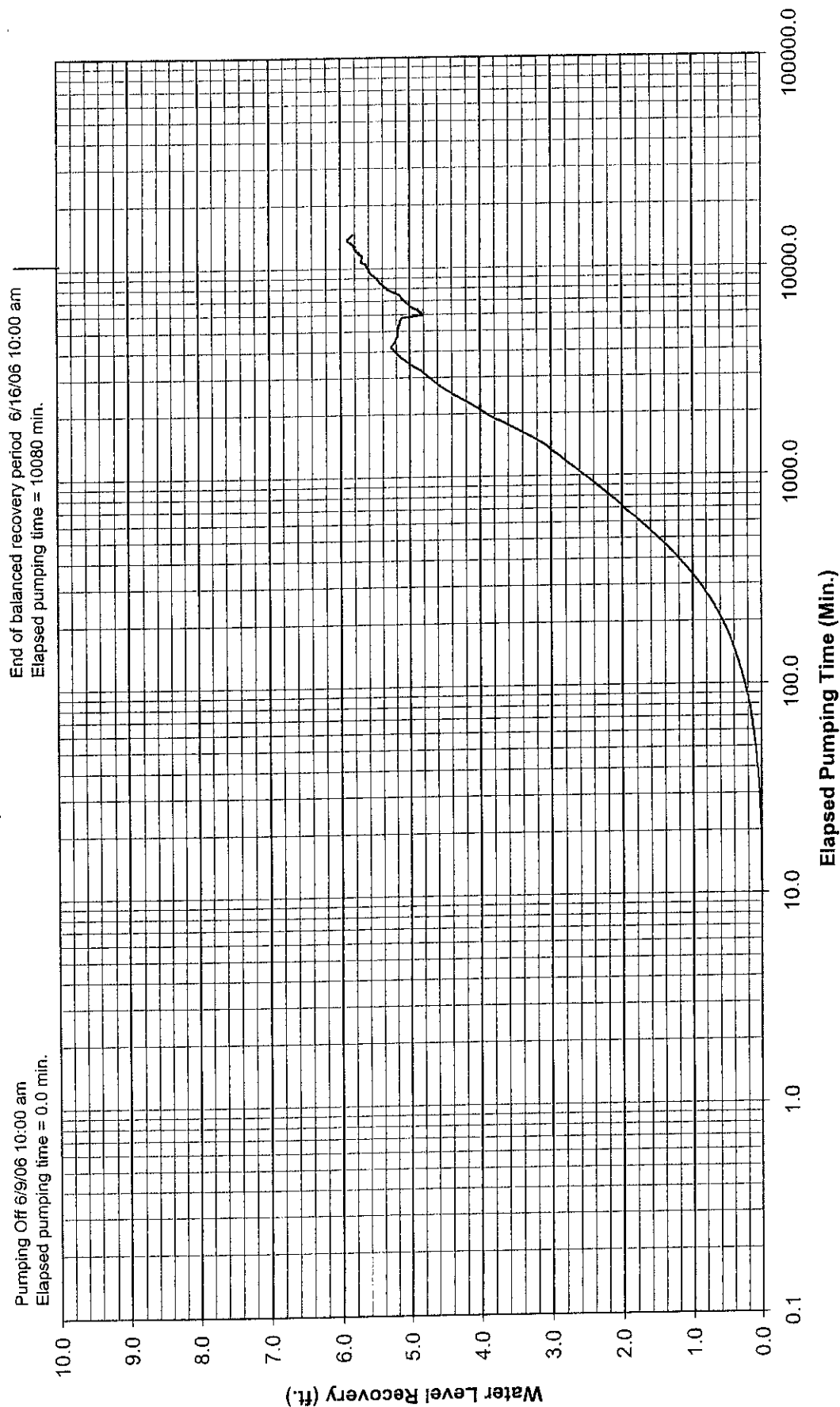
Monitoring Well No. 12 (UWI 1B North)  
Drawdown Semi-log Plot  
City of Eagle Aquifer Test

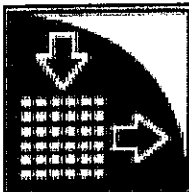
Pumping On 6/2/06 10:00 am  
Elapsed pumping time = 0.0 min.

Pump Off 6/9/06 10:00 am  
Elapsed pumping time = 10080 min.



**Monitoring Well No. 12 (UWI 1B North)**  
**Recovery Semi-log Plot**  
**City of Eagle Aquifer Test**





City, State/Province  
Address  
Contact Info  
Company Name

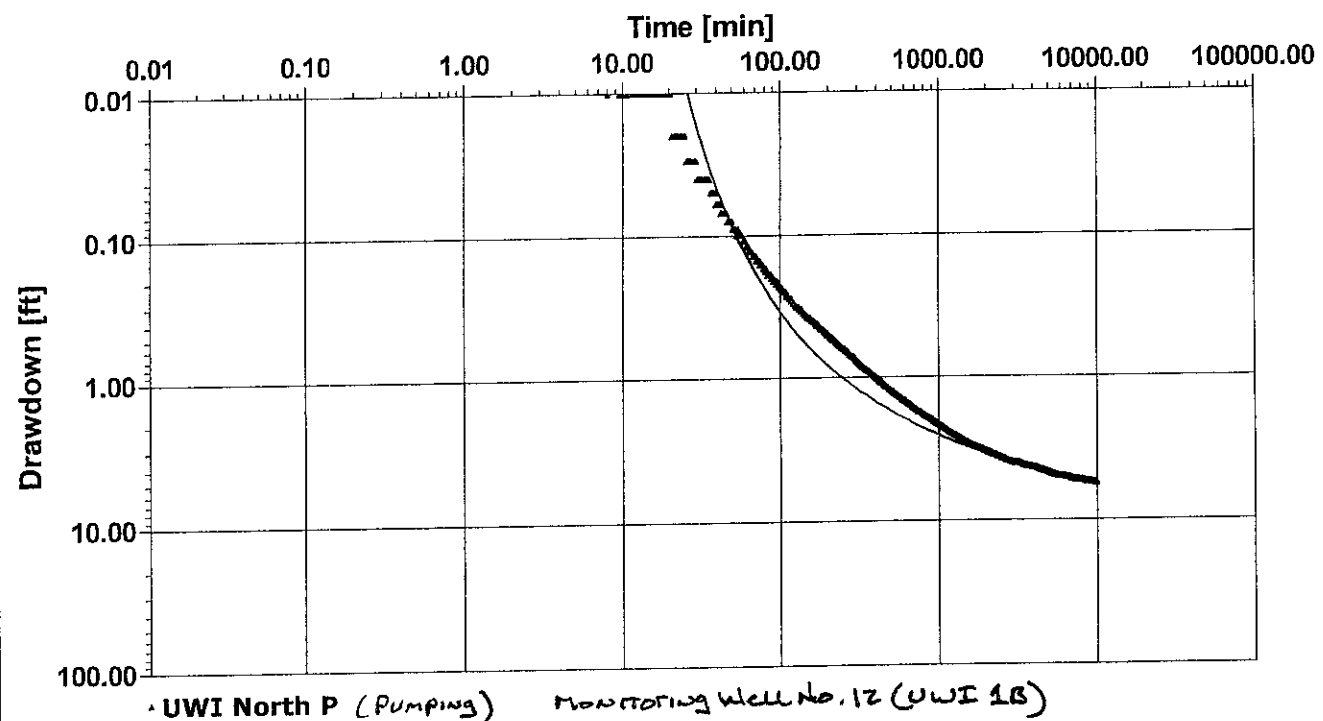
Pumping Test Analysis Report

Project: Eagle Aquifer Test

Number:

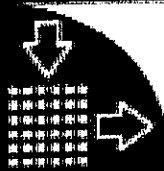
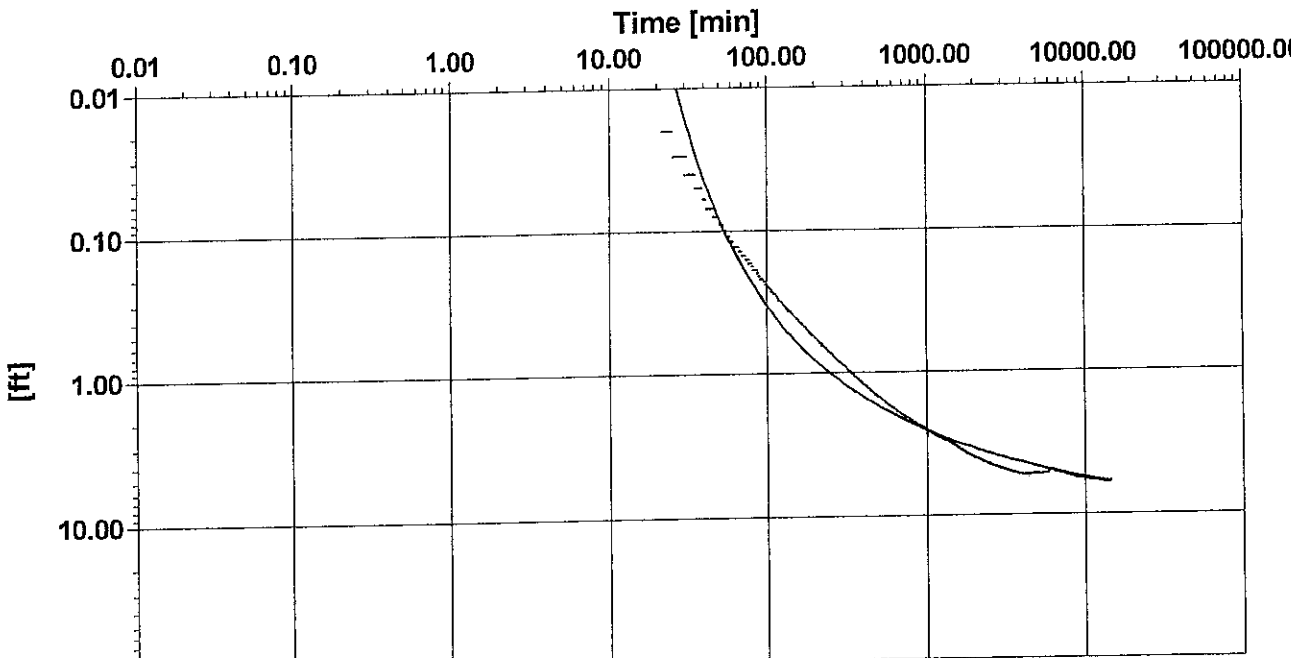
Client:

Location:	Pumping Test: Drawdown	Pumping well: Test Well No 2 P
Test conducted by:		Test date: 6/30/2006
Analysis performed by:	New analysis 1	Date: 6/30/2006
Aquifer Thickness: 100.00 ft	Discharge rate: 1580 [U.S. gal/min]	



Calculation after Theis

Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]	
UWI North P	$1.80 \times 10^4$	$1.80 \times 10^2$	$7.90 \times 10^{-4}$	2405.68	

	<b>City, State/Province</b>		<b>Pumping Test Analysis Report</b>	
	Address		Project: Recovery Eagle Aquifer Test	
	Contact Info		Number:	
	Company Name		Client:	
Location:		Pumping Test: Recovery Eagle Aquifer Test		Pumping well: TestwellNo2 R
Test conducted by:				Test date: 6/30/2006
Analysis performed by:		recovery		Date: 6/30/2006
Aquifer Thickness: 100.00 ft		Discharge rate: 1580 [U.S. gal/min]		
				
UIWNorth R (RECOVERY)      MONITORING Well No. 12 (UW11B)				
Calculation after Theis				
Observation well	Transmissivity [ft <sup>2</sup> /d]	K [ft/d]	Storage coefficient	Radial distance to PW [ft]
UIWNorth R	$1.80 \times 10^4$	$1.80 \times 10^2$	$8.00 \times 10^{-4}$	2405.68

## **Appendix C.9**

### **BAROMETER**

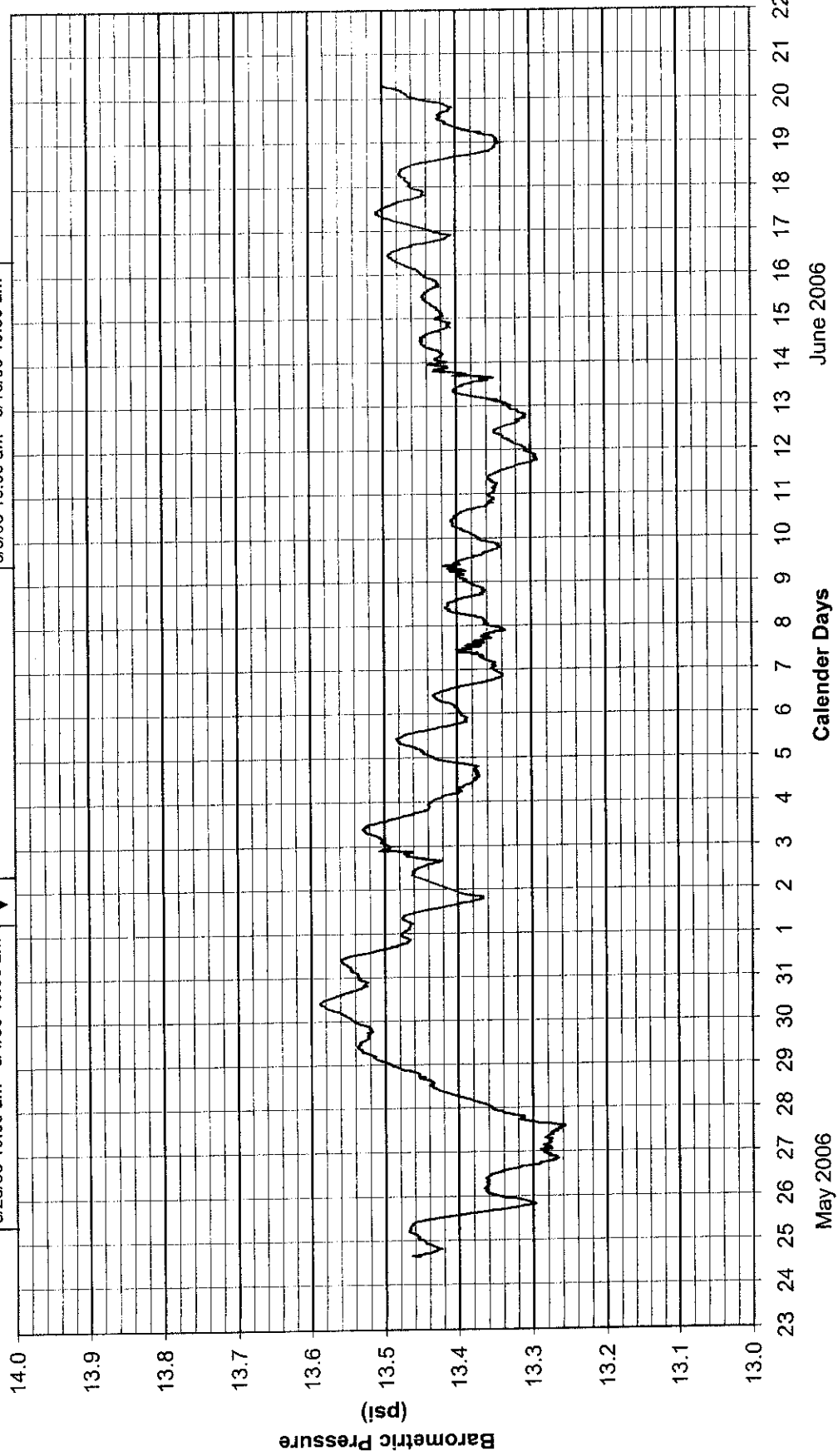
**Barometric Presures Monitoring  
Located at Test Well No. 2 (Eaglefield)  
City of Eagle Aquifer Test**

Aborted pumping period  
6/1/06 10:00 am - 6/1/06 3:45 pm

7-day pre-pumping period  
5/25/06 10:00 am - 6/1/06 10:00 am

7-day pumping period  
6/2/06 10:00 am - 6/9/06 10:00 am

7-day recovery period  
6/9/06 10:00 am - 6/16/06 10:00 am



# **APPENDIX D**

## **MONITORING WELL SURVEY DATA**

**HOLLADAY ENGINEERING COMPANY**32 N. MAIN PAYETTE, ID 83661 (208) 642-3304 FAX (208) 642-2159 email: [hec@holladayengineering.com](mailto:hec@holladayengineering.com)**MONITORING WELLS**

OWNER : CITY OF EAGLE

DATE: 6/27/06

PROJECT : MONITORING WELL ELEVATIONS

PROJECT: EG061204

AREA : T.4N., R.1W., S. 11

PAGE:

**COORDINATES:**

SURVEY PT. NO.	WELL	NORTHING	EASTING	LATITUDE	LONGITUDE	ELLIP.
1001	MW4	744362.8	2441586.0	43° 42' 24.54111" N	116° 26' 37.01283" W	2445.52
1002	MW6	744588.6	2446816.4	43° 42' 27.19661" N	116° 25' 25.84087" W	2469.58
1006	MW7	744008.1	2447329.0	43° 42' 21.50594" N	116° 25' 18.79939" W	2464.49
1010	MW9	743615.2	2445294.5	43° 42' 16.44486" N	116° 25' 58.13148" W	2454.54
1011	MW10	743595.6	2445295.1	43° 42' 16.45073" N	116° 25' 57.67684" W	2454.22
1012	MW11	739778.3	2448944.4	43° 41' 39.86635" N	116° 24' 56.34880" W	2462.67
1015	MW12	739778.3	2448944.4	43° 41' 39.86635" N	116° 24' 56.34880" W	2462.67
1008	TW1	742602.3	2446166.4	43° 42' 07.53036" N	116° 25' 34.46777" W	2456.70
1007	TW2	741256.3	2447040.0	43° 41' 54.30943" N	116° 25' 22.42992" W	2457.89

**ELEVATIONS: GROUND ELEVATIONS (NOT TOP CASING ELEVATIONS)**

WELL	ELEVATION	DESC.
MW4	2501.86	(N) HUB
MW6	2525.84	(E) HUB
MW7	2520.74	(NW) HUB
MW9	2510.83	(N) HUB
MW10	2510.52	(N) HUB
MW11	2518.83	EAST (N) RIM
MW12	2518.83	EAST (N) RIM
TW1	2512.97	(N) HUB
TW2	2514.15	

**GENERAL NOTES:**

VERTICAL: NAVD88 - BASED ON MEASURED ELLIPSOID HEIGHT AND  
CALCULATED GEOID SEPARATION (GEOID 03).

HORIZONTAL: IDAHO STATE PLANE GRID (IDAHO WEST ZONE - NAD83) -  
GROUND DISTANCES SHOWN  
USING AVERAGE COMBINED FACTOR OF 0.99986746067  
TO CONVERT GRID TO GROUND DISTANCES